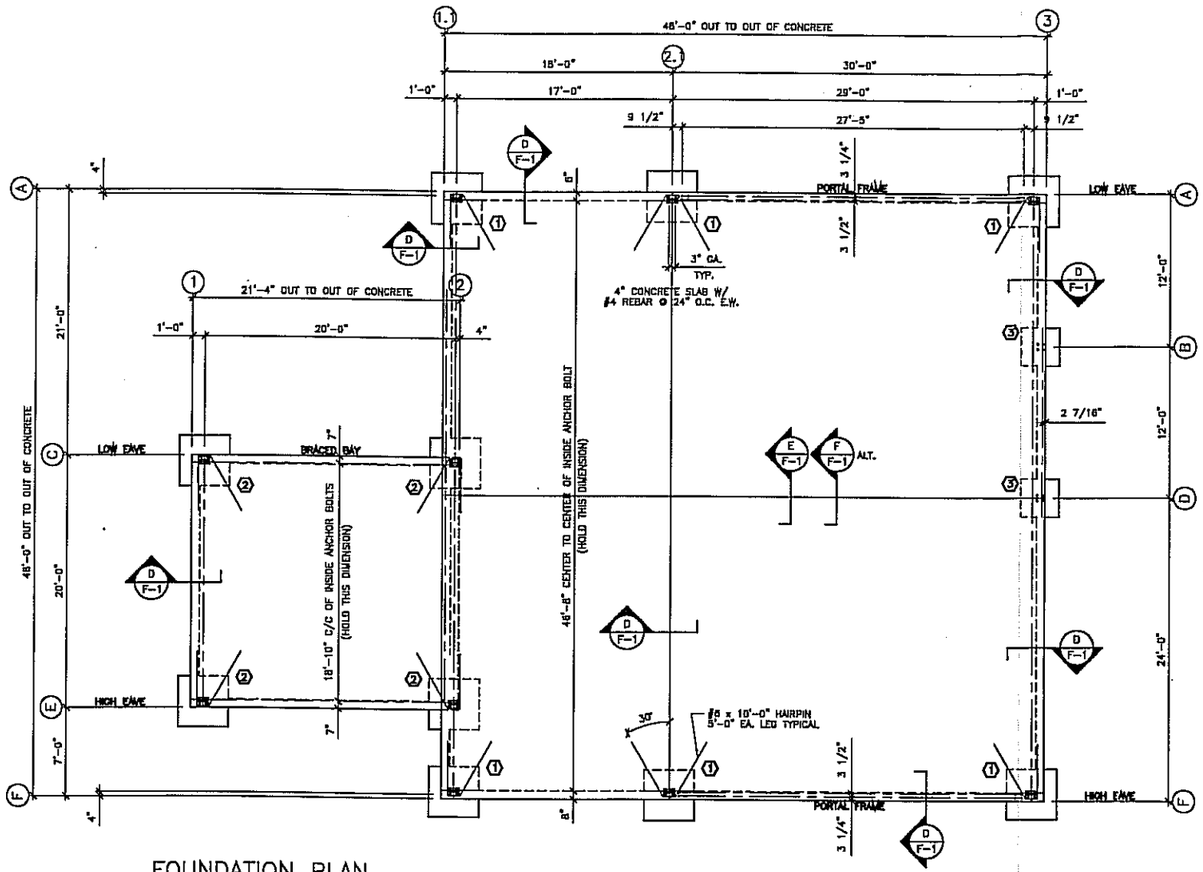


FOUNDATION PLAN

NO SCALE:

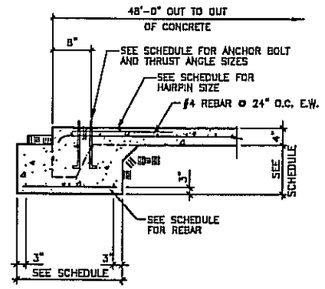
FOOTING SCHEDULE

TYPE	SIZE	DEPTH	ANCHOR	PROJ.	REBAR	THRUST ANGLE	HAIRPIN
1	4'-3" X 4'-3"	2'-0"	DIA- (4) 3/4"ø LGH- 18" GA- 3 1/2"	2 1/2"	#4 BARS @ 12" O.C. E.W.	3 X 3 X 3/16 X 0'-8" LONG	#8 BAR X 10'-0" LONG
2	2'-0" X 2'-0"	2'-0"	DIA- (4) 3/4"ø LGH- 18" GA- 3 1/2"	2 1/2"	#4 BARS @ 12" O.C. E.W.	3 X 3 X 3/16 X 0'-8" LONG	#4 BAR X 10'-0" LONG
3	2'-0" X 2'-0"	2'-0"	DIA- (4) 3/4"ø LGH- 18" GA- 3 1/2"	2 1/2"	#4 BARS @ 12" O.C. E.W.	NONE	NONE

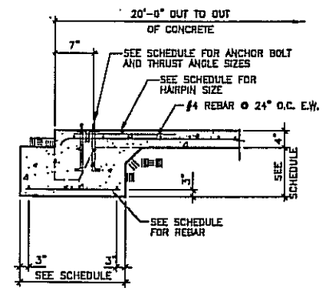
NOTE: FOR ANCHOR BOLT SIZE AND LOCATION REFER TO ANCHOR BOLT PLAN PREPARED BY HCI STEEL BUILDINGS COMPANY JOB #2791

CONCRETE NOTES:

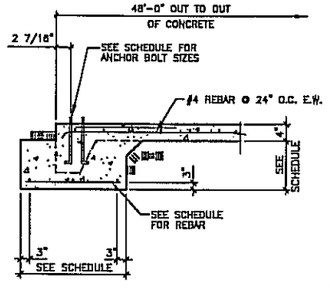
1. ALL CONCRETE SHALL WITHSTAND 2500 LBS. PER SQUARE INCH ULTIMATE COMPRESSIVE STRESS AT 28 DAYS.
2. ALL LOAD BEARING FOOTING SHALL BE A MINIMUM OF ONE FOOT (1'-0") BELOW NATURAL GRADE. ALLOWABLE SOIL BEARING PRESSURE IS 1000 LBS. PER SQUARE FOOT PER TABLE 18-1-A 1997 U.B.C.
3. CONTRACTOR SHALL INFORM THE ENGINEER OF ANY DISCREPANCIES, OMISSIONS OR ERRORS ON THE PLAN, BEFORE CONSTRUCTION, OTHERWISE, IT SHALL BE DONE AS INTENDED BY THE ENGINEER.
4. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION SUPERVISION OR DEVIATION FROM THESE PLANS WITHOUT PRIOR WRITTEN APPROVAL.
5. ALL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 40. LAP A MINIMUM OF 40 DIAMETER AT SPLICES.
6. ALL ANCHOR BOLTS SHALL PROJECT FROM THE SAME ELEVATIONS. ALL ANCHOR BOLTS TO CONFORM WITH ASTM A-307.
7. ALL CONSTRUCTION SHALL COMPLY WITH THE U.B.C. LATEST EDITION AS AMENDED BY THE LOCAL AGENCY HAVING JURISDICTIONS.
8. DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS ON DRAWINGS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS.
9. ANY ENGINEERING DESIGN PROVIDED BY OTHERS MUST BE SUBMITTED FOR REVIEW AND SHALL BEAR THE STAMP AND SIGNATURE OF A REGISTERED ENGINEER.
10. CONTRACTOR SHALL VERIFY ANCHOR BOLT SIZES; LOCATIONS AND DIMENSIONS WITH METAL BUILDING MANUFACTURER ANCHOR BOLT SETTING PLAN PRIOR TO PLACING OF ANCHOR BOLTS IN CONCRETE.
11. ANCHOR BOLTS MUST BE TIED IN PLACE PRIOR TO FOUNDATION INSPECTION.
12. FOOTING SHALL BE CENTERED ON CENTERLINE OF COLUMN ABOVE UNLESS OTHERWISE NOTED.
13. THE MINIMUM DEPTH REQUIREMENTS AND LOCAL FROST LINE REQUIREMENTS MAY SUPERSEDE DESIGN CALL OUTS. CONTACT THE LOCAL BUILDING DEPARTMENT FOR MINIMUM DEPTH REQUIREMENTS.



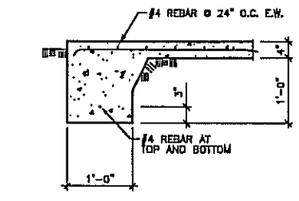
A SECTION AT FOOTING TYPE 1 — MFD-001 — SCALE: NONE



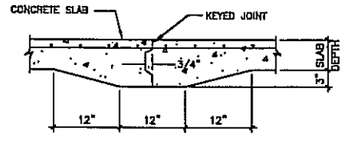
B SECTION AT FOOTING TYPE 2 — MFD-001 — SCALE: NONE



C SECTION AT FOOTING TYPE 3 — MFD-001 — SCALE: NONE

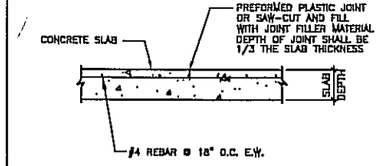


D TYPICAL SECTION AT EDGE OF SLAB — SCALE: NONE



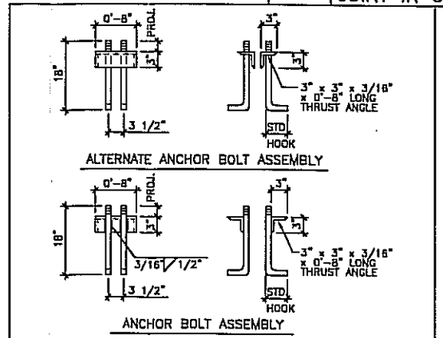
NOTE: SEE FOUNDATION PLAN FOR LOCATION OF CONSTRUCTION JOINTS.

E TYPICAL CONSTRUCTION JOINT IN CONC. SLAB — M020W — SCALE: NONE

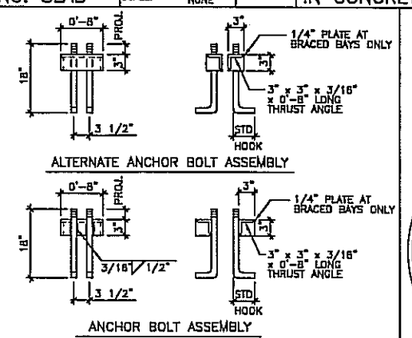


NOTE: SEE FOUNDATION PLAN FOR LOCATION OF CONTROL JOINTS.

F TYPICAL CONTROL JOINT IN CONCRETE SLAB — M021W — SCALE: NONE



G ANCHOR BOLT ASSEMBLY AT RIGID FRAME — — SCALE: NONE



H ANCHOR BOLT ASSEMBLY AT BRACED BAY — — SCALE: NONE

REGISTERED PROFESSIONAL ENGINEER
MANOS BOROS
54869
EXP.6/30/2006
03-22-2005
CIVIL
STATE OF CALIFORNIA

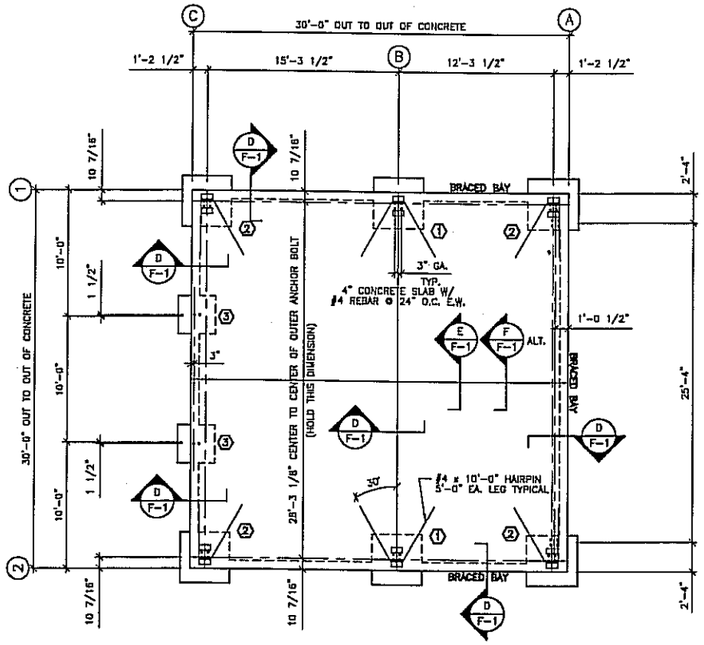
ZJS Engineering Services Inc.
300 N. MAIN AVE. SUITE A
CLOVIS, CA 93611
PHONE: (559) 974-4150 FAX:(559) 974-4163

PROJECT: DUNLAP 48
LOCATION: SQUAW VALLEY, CA
TITLE: FOUNDATION PLAN

REVISIONS	DATE	CHK'D BY

SUBMITTAL DATE: 03-01-05
DRAWN BY: DW
CHECKED BY:
SCALE: NONE
JOB NO.: 19006-05
SHEET NO.: F-1
1 OF 1 SHEETS

10 of 37



FOUNDATION PLAN

NO SCALE:

FOOTING SCHEDULE							
TYPE	SIZE	DEPTH	ANCHOR	PROJ.	REBAR	THRUST ANGLE	HAIRPIN
1	2'-9" X 2'-9"	2'-6"	DIA- (4) 3/4"ø LGH- 18" GA- 3"	2 1/2"	#4 BARS @ 12" O.C. E.W.	3 X 3 X 3/16 X 0'-8" LONG	#4 BAR X 10'-0" LONG
2	2'-6" X 2'-6"	2'-6"	DIA- (4) 3/4"ø LGH- 18" GA- 3"	2 1/2"	#4 BARS @ 12" O.C. E.W.	3 X 3 X 3/16 X 0'-8" LONG	#4 BAR X 10'-0" LONG
3	2'-0" X 2'-0"	2'-0"	DIA- (2) 1/2"ø LGH- 18" GA- 4"	1 1/2"	#4 BARS @ 12" O.C. E.W.	NONE –	NONE –

NOTE: FOR ANCHOR BOLT SIZE AND LOCATION REFER TO ANCHOR BOLT PLAN PREPARED BY HCI STEEL BUILDINGS COMPANY JOB #2792

CONCRETE NOTES:

1. ALL CONCRETE SHALL WITHSTAND 2500 LBS. PER SQUARE INCH ULTIMATE COMPRESSIVE STRESS AT 28 DAYS.
2. ALL LOAD BEARING FOOTING SHALL BE A MINIMUM OF ONE FOOT (1'-0") BELOW NATURAL GRADE. ALLOWABLE SOIL BEARING PRESSURE IS 1000 LBS. PER SQUARE FOOT PER TABLE 18-I-A 1997 U.B.C.
3. CONTRACTOR SHALL INFORM THE ENGINEER OF ANY DISCREPANCIES, OMISSIONS OR ERRORS ON THE PLAN, BEFORE CONSTRUCTION. OTHERWISE, IT SHALL BE DONE AS INTENDED BY THE ENGINEER.
4. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION SUPERVISION OR DEVIATION FROM THESE PLANS WITHOUT PRIOR WRITTEN APPROVAL.
5. ALL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 40. LAP A MINIMUM OF 40 DIAMETER AT SPLICES.
6. ALL ANCHOR BOLTS SHALL PROJECT FROM THE SAME ELEVATIONS. ALL ANCHOR BOLTS TO CONFORM WITH ASTM A-307.
7. ALL CONSTRUCTION SHALL COMPLY WITH THE U.B.C. LATEST EDITION AS AMENDED BY THE LOCAL AGENCY HAVING JURISDICTIONS.
8. DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS ON DRAWINGS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS.
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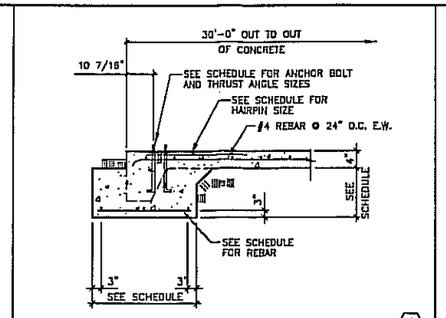
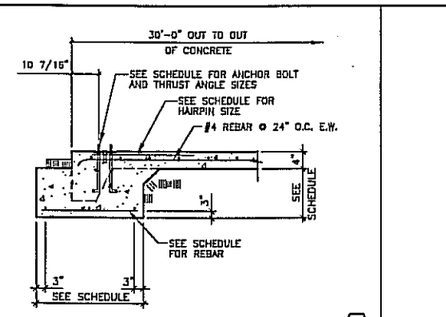
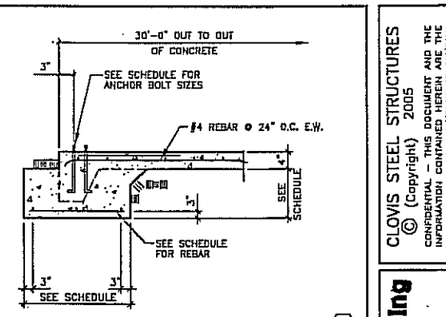
13. THE MINIMUM DEPTH REQUIREMENTS AND LOCAL FROST LINE REQUIREMENTS MAY SUPERSEDE DESIGN CALL OUTS. CONTACT THE LOCAL BUILDING DEPARTMENT FOR MINIMUM DEPTH REQUIREMENTS.



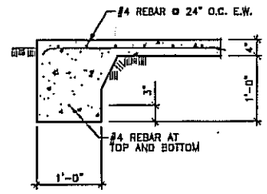
A SECTION AT FOOTING TYPE 1 — MFD-001 — SCALE: NONE



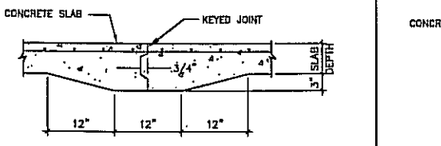
B SECTION AT FOOTING TYPE 2 — MFD-001 — SCALE: NONE



C SECTION AT FOOTING TYPE 3 — MFD-001 — SCALE: NONE

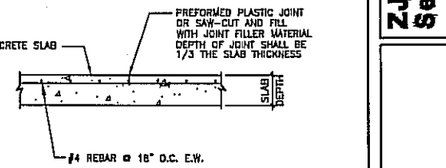


D TYPICAL SECTION AT EDGE OF SLAB — SCALE: NONE



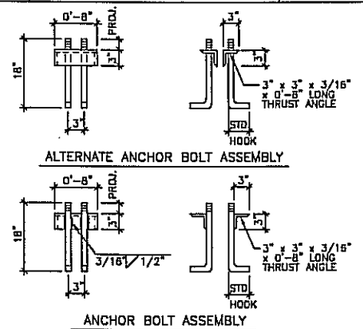
NOTE: SEE FOUNDATION PLAN FOR LOCATION OF CONSTUCTION JOINTS.

E TYPICAL CONSTRUCTION JOINT IN CONC. SLAB — M020W — SCALE: NONE

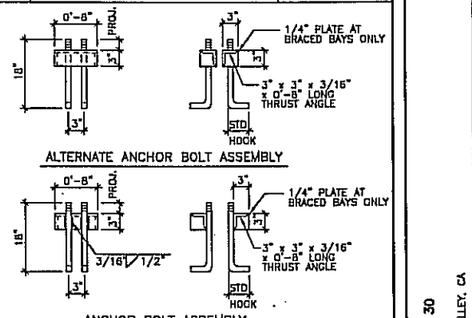


NOTE: SEE FOUNDATION PLAN FOR LOCATION OF CONTROL JOINTS.

F TYPICAL CONTROL JOINT IN CONCRETE SLAB — M021W — SCALE: NONE



G ANCHOR BOLT ASSEMBLY AT RIGID FRAME — – — SCALE: NONE



H ANCHOR BOLT ASSEMBLY AT BRACED BAY — – — SCALE: NONE

ZJS Engineering Services Inc.
700 S. VILLASPI AVE. SUITE A
ONTARIO, CA 91761 FAX:(909) 974-4153
PHONE: (909) 974-4150

PROJECT DUNLAP 30
LOCATION SQUAW VALLEY, CA.
TITLE FOUNDATION PLAN

REVISIONS	DATE	CHK BY

SUBMITTAL DATE 02-28-05
DRAWN BY DW
CHECKED BY
SCALE: NONE
JOB NO. 19005-06
SHEET NO. F-1
1 OF 1 SHEETS

REGISTERED PROFESSIONAL ENGINEER
ANOS BOROS
54869
EXP.6/30/2006
03-22-2005
CIVIL
STATE OF CALIFORNIA

Sheet 24 of 37

SHEETING SCHEDULE

	TYPE	COLOR	GA.
ROOF PANEL	HR	GALVANIZED	26
WALL PANEL	HR		26
RAKE/EAVE TRIM			26
WALL TRIM			26
CORNER TRIM			26
JAMB/HEAD TRIM			26

SCREWS SCHEDULE

MARK	SIZE	LENGTH
FRAMING SCREW W/O WASHER	12-14	1"
ROOF FRAMING SCREW	12-14	1 1/2"
WALL FRAMING SCREW	12-14	1 1/2"
TRIM FRAMING SCREW	12-14	1"
STITCH SCREW	1/4"-14	7/8"
STRUCTURAL SCREW	12-24	1 1/4"
WAFER HEAD SCREWS	8-18	2 3/8"
WAFER HEAD SCREWS	10-24	1"

FRAMED OPENING SCHEDULE

QNTY.	DESCRIPTION
1	10'-0" x 10'-0" RUD (DOOR BY OTHERS)

MAN DOOR SCHEDULE

QNTY.	MARK	
1	3070M	SOLID

WINDOW SCHEDULE

QNTY.	DESCRIPTION
2	3030 HALF SLIDER

INSULATION

	THICKNESS	BY HCI	NOT BY HCI
ROOF			
WALL			
ROOF T-BLOCK			
WALL T-BLOCK			

SKYLIGHT/WALLITE PANELS SCHEDULE

PROFILE	QNTY.	LENGTH
HUSKY HI RIB	6	11'-0"
WEATHER SPAN		

GENERAL NOTES:

1.) MATERIALS

	ASTM DESIGNATION
HOT ROLLED BAR	A529 GRADE 55 Fy = 55 ksi MIN.
STRUCTURAL WIDE FLANGE SHAPES	A992 Fy = 50 ksi MIN.
STRUCTURAL CHANNEL/ANGLE	A36 Fy = 36 ksi MIN.
STRUCTURAL STEEL SHEET	A570 GRADE 50 Fy = 50 ksi MIN.
STRUCTURAL STEEL PLATE	A572 GRADE 50 Fy = 50 ksi MIN.
COLD FORMED SHAPES	A570 GRADE 55 Fy = 55 ksi Min.
COLD FORMED SHAPES (GALV.)	A653 55M GRADE 55 CLASS 1 G40 Fy = 55 ksi Min.
ROOF & WALL SHEETING (SCREW DOWN ROOF)	A653 SS GRADE 50 CLASS 1 OR 2 G90 26 Ga. Fy = 50 ksi Min.
ROOF & WALL SHEETING (STANDING SEAM ROOF)	A653 SS GRADE 40 G90 24 Ga. Fy = 40 ksi Min.
BOLTS	A307 & A325

HCI STEEL BUILDING SYSTEMS RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIALS.

2.) A325 BOLT TIGHTENING REQUIREMENTS.
ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE. HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE "TURN OF THE NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION OF AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE INSTALLED WITHOUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT" METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT ONLY.

3.) ALL STRUCTURAL STEEL SHALL RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

4.) PRE-FORMED CLOSURE STRIPS ARE PROVIDED AT THE EAVE BENEATH THE ROOF SHEETS, AT THE ENDWALL BETWEEN THE RAKE TRIM AND THE ENDWALL SHEETS AND AT THE SIDEWALL BETWEEN THE EAVE TRIM AND THE SIDEWALL SHEETS.

5.) ALL BRACING SHOWN AND PROVIDED BY HCI STEEL BUILDING SYSTEMS FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING, AND TO PROCURE AND INSTALL AS NEEDED.

6.) ERECTOR NOTE:
OSHA 2001 STEEL ERECTION STANDARDS – SUBPART "R" 1926.758(e) , REQUIRES THAT, "IN GIRT AND EAVE STRUT-TO-FRAME CONNECTIONS, WHEN GIRTS OR EAVE STRUTS SHARE COMMON CONNECTION HOLES, AT LEAST ONE BOLT WITH ITS WRENCH TIGHT NUT REMAIN CONNECTED TO THE FIRST MEMBER SO THAT THE GIRT OR EAVE STRUT IS ALWAYS SECURED AGAINST DISPLACEMENT. HCI HAS PROVIDED AN ADEQUATE QTY OF THINHEAD BOLTS FOR THIS PURPOSE. HOWEVER, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO UNDERSTAND AND FOLLOW OSHA SAFETY REGULATIONS AND PROCEDURES.

CUSTOMER RESPONSIBILITIES

1.) IT IS THE RESPONSIBILITY OF THE ERECTOR TO PERFORM THE WORK IN A SAFE MANNER AND TO OBEY ALL THE SAFETY CODES.

2.) IT IS THE RESPONSIBILITY OF THE CUSTOMER TO ENSURE THAT ALL PROJECT PLANS, SPECIFICATIONS AND APPLIED DESIGN LOADS FULLY MEET THE PROPOSED INTENT AS WELL AS COMPLY WITH ALL APPLICABLE REQUIREMENTS OF ANY GOVERNING AUTHORITES, AND OBTAIN THE APPROPRIATE APPROVALS AND/OR PERMITS AS MAY BE REQUIRED FROM CITY, COUNTY, STATE OR FEDERAL AGENCIES.

3.) THE BUILDING WILL BE SUPPLIED AS THE HCI DRAWINGS INDICATE. IT IS THE RESPONSIBILITY OF THE CUSTOMER TO ENSURE THE THE BUILDING BEING SUPPLIED MEETS THE PROJECT REQUIREMENTS.

4.) IT IS THE RESPONSIBILITY OF THE CUSTOMER FOR OVERALL PROJECT COORDINATION, INTERFACE, COMPATIBILITY AND DESIGN CONCERNING ANY MATERIALS NOT SUPPLIED BY HCI STEEL BUILDING SYSTEMS, INC. WITH THE BUILDING THE TO BE SUPPLIED BY HCI STEEL BUILDING SYSTEMS, INC. THE SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BY THE CUSTOMER PRIOR TO RELEASE FOR FABRICATION, OR THE DESIGN ASSUMPTIONS OF HCI STEEL BUILDING SYSTEMS WILL GOVERN.

5.) IT IS THE RESPONSIBILITY OF THE CUSTOMER TO VERIFY ALL DESIGN CRITERIA INCLUDING DEFLECTIONS AND TO NOTIFY HCI STEEL BUILDING SYSTEMS OF ANY DISCREPANCIES OR INCONSISTENCIES PRIOR TO RELEASE FOR FABRICATION.

6.) THE CUSTOMER IS RESPONSIBLE FOR THE ACCURATE SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH THE "FOR CONSTRUCTION" DRAWINGS ONLY. (SEE ANCHOR BOLT PLAN FOR ADDITIONAL FOUNDATION AND ANCHOR BOLT NOTES)

7.) UNLESS SPECIFICALLY STATED OTHERWISE WITHIN THE HCI STEEL BUILDING SYSTEMS PURCHASE AGREEMENT, THE FOLLOWING SHALL GOVERN:

a.) THIS BUILDING DRAWING PACKAGE AND ANY ACCOMPANYING CALCULATIONS REPRESENT THE PROJECT AS IT WILL SUPPLIED. THE BUILDING WILL BE SUPPLIED EXACTLY AS THE DRAWINGS INDICATE.

b.) ANY FIELD MODIFICATIONS, ALTERATIONS, OR THE ATTACHMENT OF ANY EQUIPMENT OR COMPONENT TO THIS HCI STEEL BUILDING SYSTEM, OR THE OMITTED INSTALLATION OF ANY PARTS OR PIECES, WITHOUT THE EXPRESS WRITTEN APPROVAL OF HCI STEEL BUILDING SYSTEM, INC. SHALL VOID ANY AND ALL WARRANTIES.

CUSTOMER APPROVAL:

ALL INFORMATION CONTAINED HEREIN HAS BEEN REVIEWED AND FOUND TO BE CORRECT AND CONSISTENT WITH MY INTENT AND PURPOSE. I REQUEST THAT H C I STEEL BUILDING SYSTEMS PROCEED WITH FABRICATION. I UNDERSTAND AND ACCEPT ALL CUSTOMER RESPONSIBILITIES.

CHECK ONE

- REVISE AND RESUBMIT
- APPROVED FOR FABRICATION AS NOTED
- APPROVED FOR FABRICATION

SIGNED: DATE:

BASIC BUILDING SIZE: SPAN 30'-0" x LENGTH 30'-0" x EAVE HT. 13'-0"
ROOF PITCH: 1.0:12
DESIGN CODE/ED.: CBC 01 EXPOSURE: C SEISMIC ZONE: 3
LIVE LOAD: 20R psf. DEAD LOAD: 2.5 psf. SNOW LOAD: 0 psf. WIND LOAD: 75 mph.
CLOSURE: Enclosed COLLATERAL LOAD: 1 psf ADDITIONAL LOAD:

REV.	DATE	REVISION
A	1/19/05	FOR APPROVAL

HCI STEEL BUILDING SYSTEMS, INC.
18520 67th AVE. N.E.
ARLINGTON, WA 98223
(360) 435-8871
FAX (360) 435-9267

BUILDING DRAWING PACKAGE

CUSTOMER: CLOVIS STEEL STRUCTURES
ADDRESS: 8350 E. SHAW AVE.
CLOVIS, CA 93611
PHONE: (559) 298-4145 FAX: (559) 323-5302
PROJECT: DUNLAP 30
LOCATION: SQUAW VALLEY, CA 93675

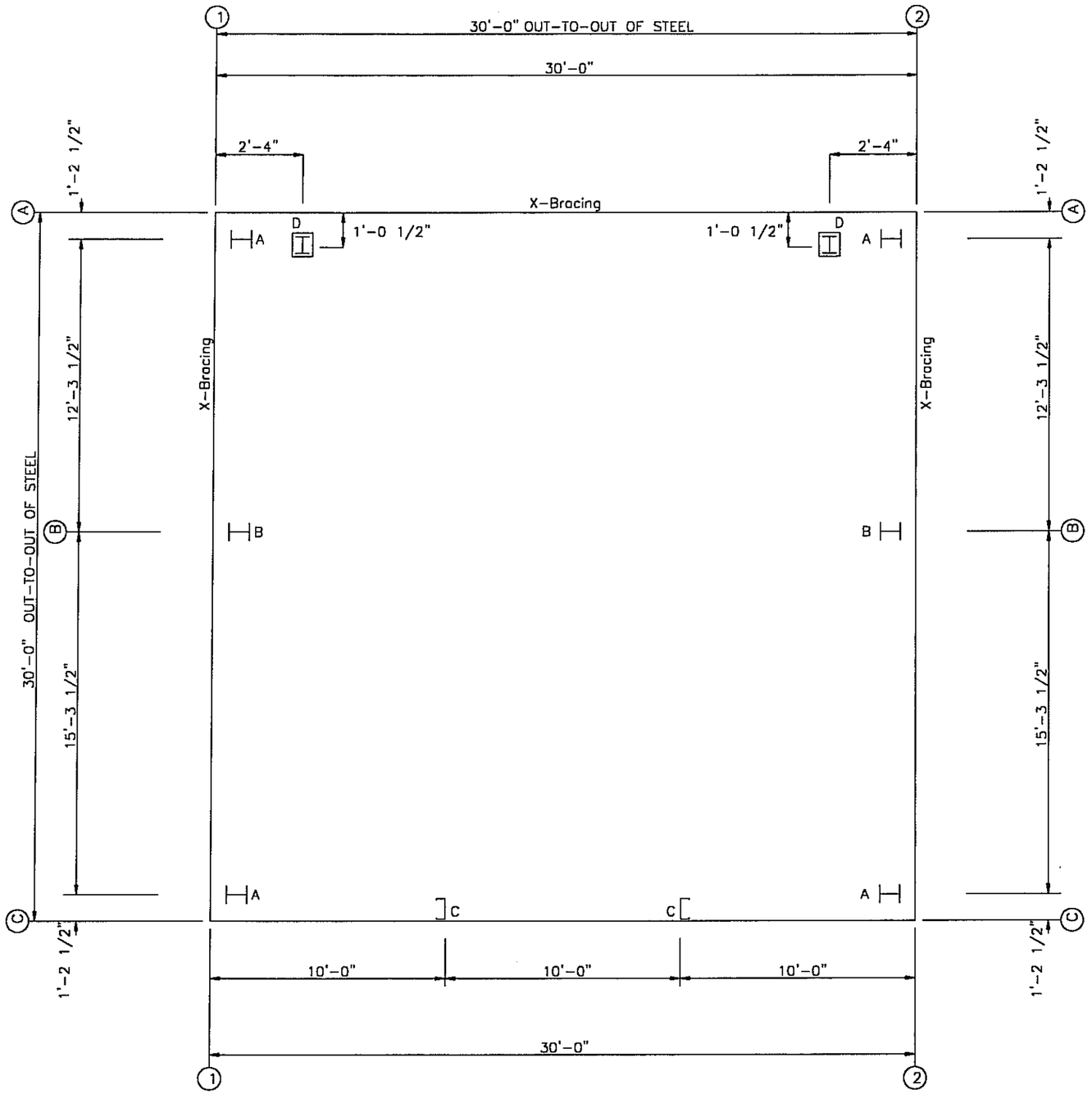
DRAWING RECORD

DESCRIPTION	PAGE NO.
ANCHOR BOLT PLAN	PEB-1 & 2
ROOF FRAMING PLAN	PEB-3
SIDEWALL ELEVATIONS	PEB-4 & 5
ENDWALL ELEVATIONS	PEB-6 & 7
CROSS SECTION	PEB-8
BRIDGING DETAILS	PEB-3
B.F.E.W. DETAILS	PEB-4
FRAMED OPENINGS	PEB-9
DOOR DETAILS	PEB-10
WINDOW DETAILS	PEB-10
ROOF DETAILS	PEB-12
WALL DETAILS	PEB-11
GUTTER DETAILS	N/A
DETAILS	PEB-
DETAILS	PEB-
DETAILS	PEB-

JOB NO.: 2792

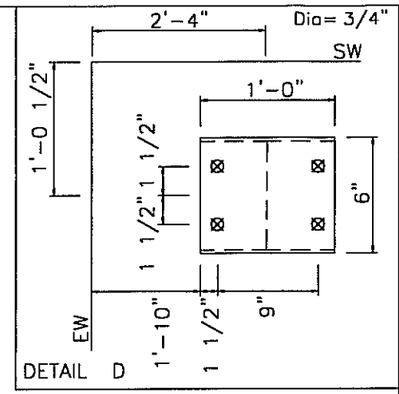
SET #2 GR 1-26-05

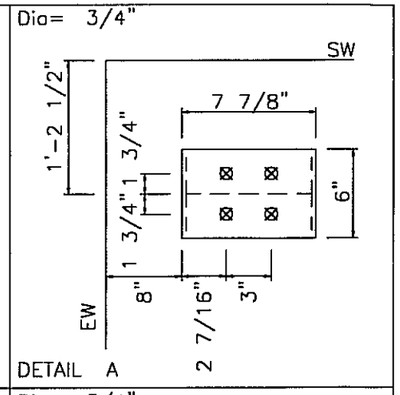
REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 6/30/06
CIVIL
STATE OF CALIFORNIA
1/19/05

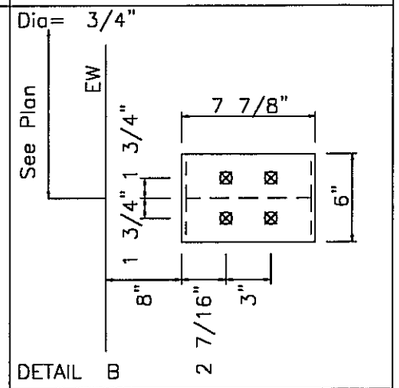


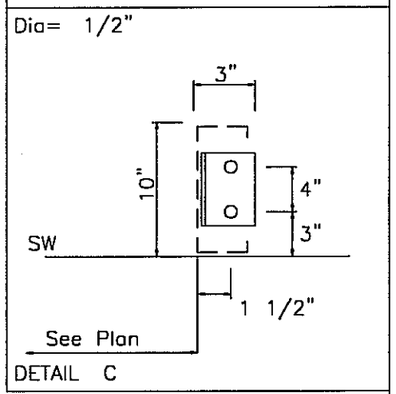
ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)









REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 6/30/06
STATE OF CALIFORNIA
11/19/05

Sheet 26 of 37

REV.	DATE	DESCRIPTION
		REVISIONS
A	11/19/05	FOR APPROVAL
		REVISIONS

HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30
CUSTOMER:	CLOVIS STEEL STRUCTURES
LOCATION:	SQUAW VALLEY, CA 93675

DRAWN:	DATE:	JOB NO.	PAGE:
JDM	1/14/05	2792	PEB-1
CHECK:	DATE:		

ENDWALL COLUMN: REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Dead Vert	Coll Vert	Live Vert	Wind-Left Horiz	Wind-Left Vert	Wind-Right Horiz	Wind-Right Vert	Out-Of-Plane Wd P Horiz	Out-Of-Plane Wd S Horiz
1	C	0.5	0.1	2.1	0.0	−1.1	0.0	−1.1	−0.9	0.9
1	B	1.0	0.3	5.4	0.0	−0.4	2.8	−5.4	−1.9	1.9
1	A	0.4	0.1	1.5	2.8	−3.3	0.0	1.7	−0.7	0.7
2	C	0.5	0.1	2.1	0.0	−1.1	0.0	−1.1	−0.9	0.9
2	B	1.0	0.3	5.4	2.8	−5.4	0.0	−0.4	−1.9	1.9
2	A	0.4	0.1	1.5	0.0	1.7	2.8	−3.3	−0.7	0.7

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Anc. Bolt No	D(in)	Base Plate (in) Wid	Len	Thk	Grout (in)
1	A	1	0.7	−0.5	2	−0.7	−0.5	4	0.750	6.000	7.875	0.375	0.0
		3	0.7	2.9	4	0.7	−2.3						
1	B	1	1.9	−1.9	2	−1.9	−1.9	4	0.750	6.000	7.875	0.375	0.0
		5	0.0	6.7									
1	C	1	0.9	−0.7	2	−0.9	−0.7	4	0.750	6.000	7.875	0.375	0.0
		5	0.0	2.7									
2	C	1	0.9	−0.7	2	−0.9	−0.7	4	0.750	6.000	7.875	0.375	0.0
		5	0.0	2.7									
2	B	1	1.9	−1.9	2	−1.9	−1.9	4	0.750	6.000	7.875	0.375	0.0
		5	0.0	6.7									
2	A	1	0.7	−0.5	2	−0.7	−0.5	4	0.750	6.000	7.875	0.375	0.0
		4	0.7	2.9	3	0.7	−2.3						

BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions (k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert
L_EW	1	A	2.8	2.5	0.4	0.3
		B	2.8	2.5	0.4	0.3
F_SW	C					
R_EW	2	B	2.8	2.5	0.4	0.3
		A	2.8	2.5	0.4	0.3
B_SW	A	2 ,1	4.0	1.4	0.7	0.2

ANCHOR BOLT SUMMARY

	Qnt	Loc	Dia (in)	Proj (in)
O	4	DJ	1/2"	2.00
⊠	24	EW	3/4"	3.00
⊠	8	WB	3/4"	3.00

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

Width (ft)	= 30.0
Length (ft)	= 30.0
Eave Height (ft)	= 13.0/13.0
Roof Slope (rise/12)	= 1.0/ 1.0
Dead Load (psf)	= 2.5
Collateral Load (psf)	= 1.0
Live Load (psf)	= 20.0
Wind Speed (mph)	= 75.0
Wind Code	= CBC 01
Exposure	= C
Closed/Open	= C
Importance - Wind	= 1.00
Importance - Seismic	= 1.00
Seismic Zone	= 3
Seismic Coeff (Ca)	= 0.33

5. Loading conditions are:
   1 DL+WR1+WS
   2 DL+WP+LnWndL
   3 DL+LL/2+WR2+WS
   4 DL+LL/2+WL2+WS
   5 DL+CL+LL

GENERAL NOTES

FOUNDATION NOTES:

1. ANCHOR BOLTS, EMBEDDED ITEMS AND FOUNDATION DESIGN ARE NOT BY HCI AND REQUIRE SPECIAL REINFORCEMENT.
2. FOUNDATION MUST BE SQUARE, LEVEL AND SMOOTH. ALL ANCHOR BOLTS MUST BE PLACED ACCURATELY IN THE POSITIONS SHOWN OR STEEL WILL NOT FIT.
3. BOTTOM OF ALL BASE PLATES TO BE AT SAME ELEVATION UNLESS NOTED.
4. COLUMN REACTIONS ARE IN THOUSANDS OF POUNDS.
5. ANCHOR BOLTS ARE TO BE TYPE A36 AND REQUIRE FLAT WASHERS.
6. REACTIONS SHOULD BE CONSIDERED SWITCHABLE WHEN LOADS ARE APPLIED FROM THE OPPOSITE SIDE.
7. ALLOWABLE CONCRETE BEARING PRESSURE USED FOR BASE PLATE DESIGN IS 1.05 KSI.

REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 6/30/06
CIVIL
STATE OF CALIFORNIA
1/19/05

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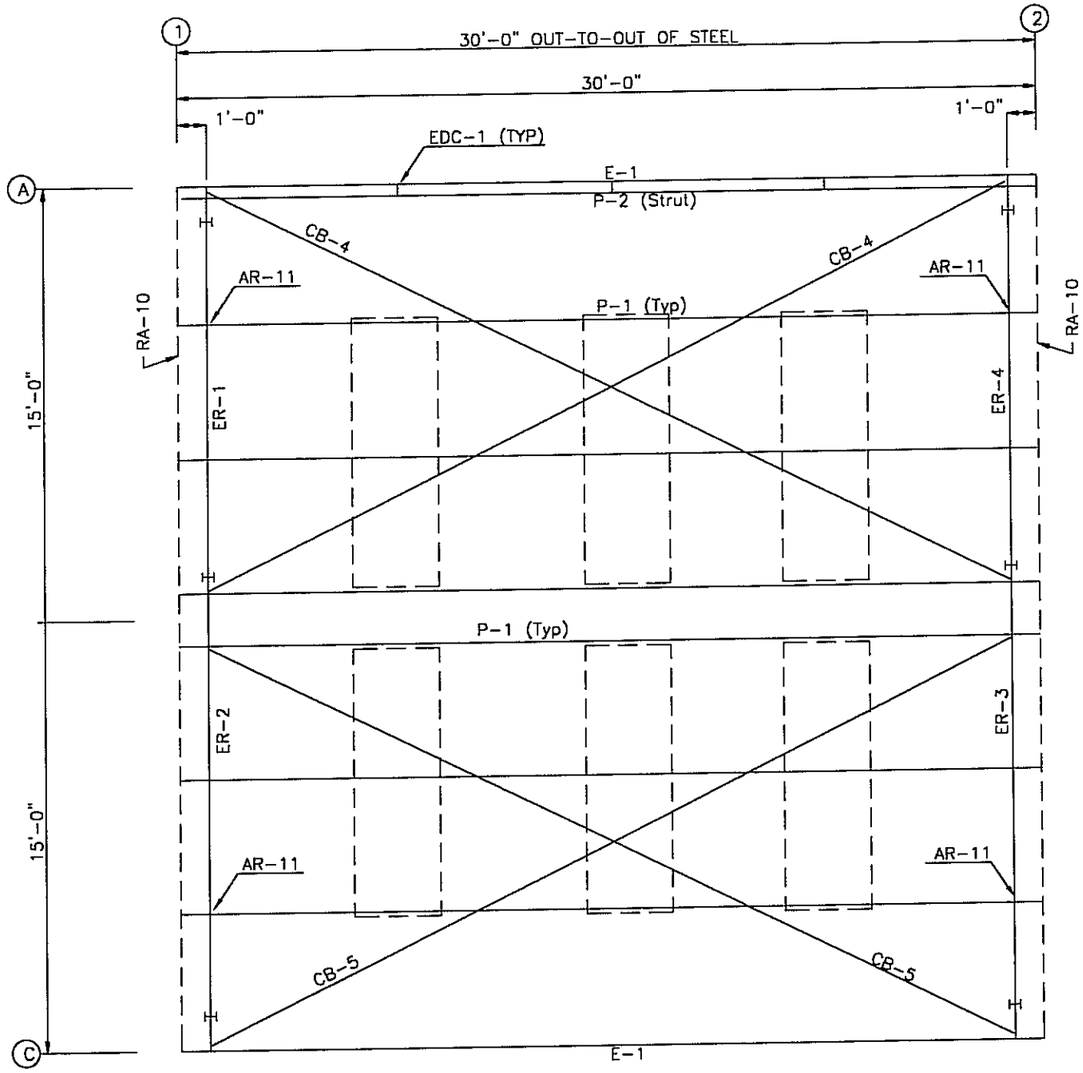
REV.	DATE	DESCRIPTION
		REVISIONS
1	1/19/05	FOR APPROVAL
		REVISIONS

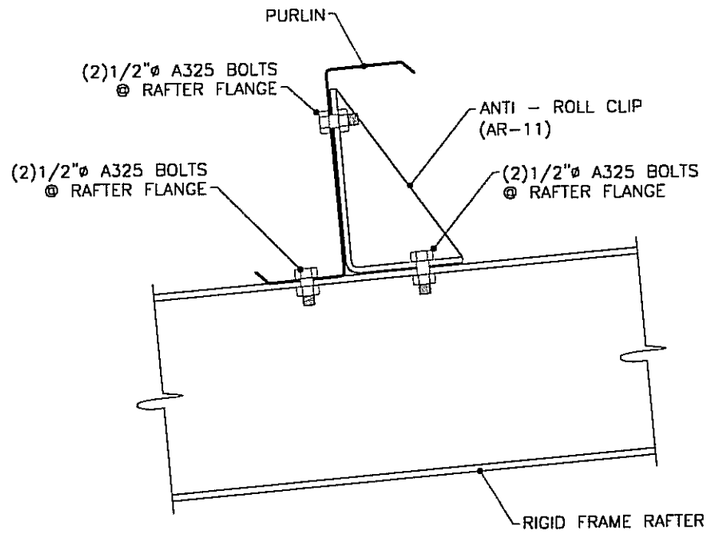
HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30	DRAWN: JDM	DATE: 1/14/05	JOB NO. 2792	PAGE: PEB-2
CUSTOMER:	CLOVIS STEEL STRUCTURES	CHECK:	DATE:		
LOCATION:	SQUAW VALLEY, CA 93675				

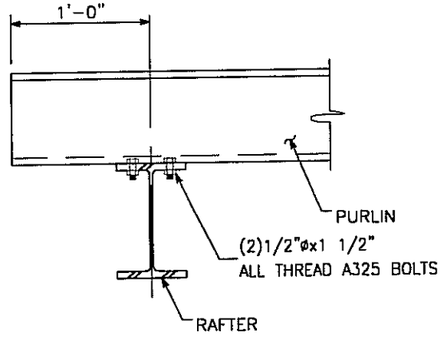
MEMBER TABLE ROOF PLAN	
MARK	SIZE
P-1	11Z25U12
P-2	11Z25U12
E-1	11Z25U12
CB-4	ROD625
CB-5	ROD500



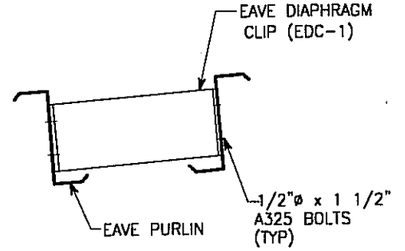
ROOF FRAMING PLAN



ANTI-ROLL CLIP DETAIL



END BAY CONN. DETAIL



DIAPHRAGM CLIP DETAIL

Sheet 28 of 37

ERECTOR NOTE:
PIECE MARKS ARE LOCATED ON THE LEFT END OF MEMBER AS DETAILED. ERECT MEMBER SO PIECE MARK IS IN POSITION SHOWN ON ERECTION DRAWINGS.

REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 12/31/06
CIVIL
STATE OF CALIFORNIA
11/19/05

REV.	DATE	DESCRIPTION
A	1/19/05	FOR APPROVAL

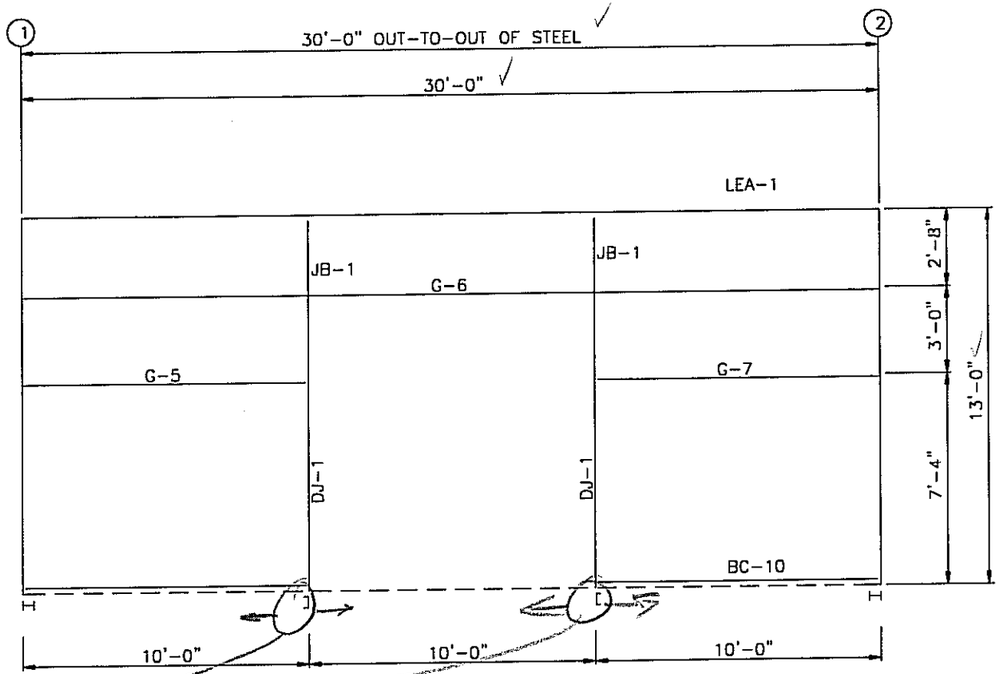
REVISIONS

HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30
CUSTOMER:	CLOVIS STEEL STRUCTURES
LOCATION:	SQUAW VALLEY, CA 93675

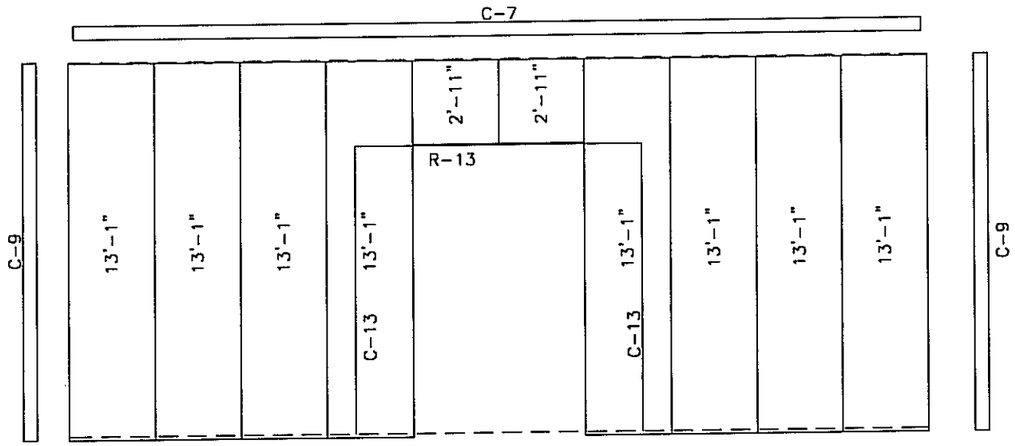
DRAWN:	DATE:	JOB NO.	PAGE:
JDM	1/14/05	2792	PEB-3
CHECK:	DATE:		

MEMBER TABLE LINE C	
MARK	SIZE
DJ-1	10C30E16
JB-1	10C30E16
G-5	10Z30U16
G-6	10Z30U12
G-7	10Z30U16



Can you adjust these?

FRONT SIDEWALL FRAMING: LINE C



FRONT SIDEWALL SHEETING & TRIM: LINE C

PANELS: 26 Ga. HUSKY HI-RIB –

Sheet 29 of 37

ERECTOR NOTE:
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REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 6/30/06
CIVIL
STATE OF CALIFORNIA
1/19/05

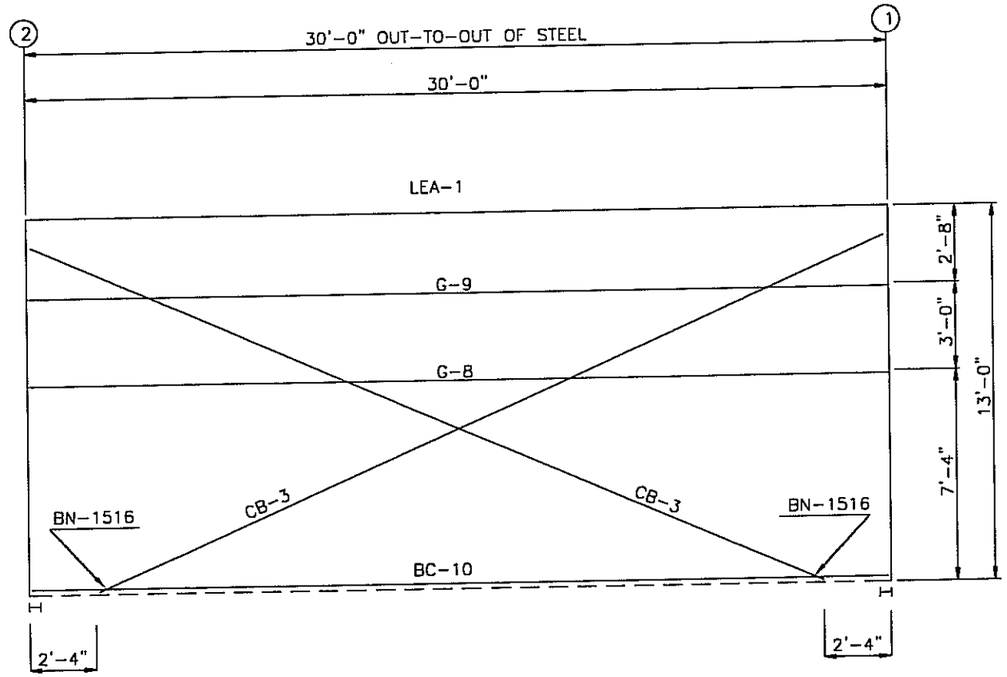
REV.	DATE	DESCRIPTION
		REVISIONS
A	1/19/05	FOR APPROVAL

HCI STEEL BUILDING SYSTEMS INC.

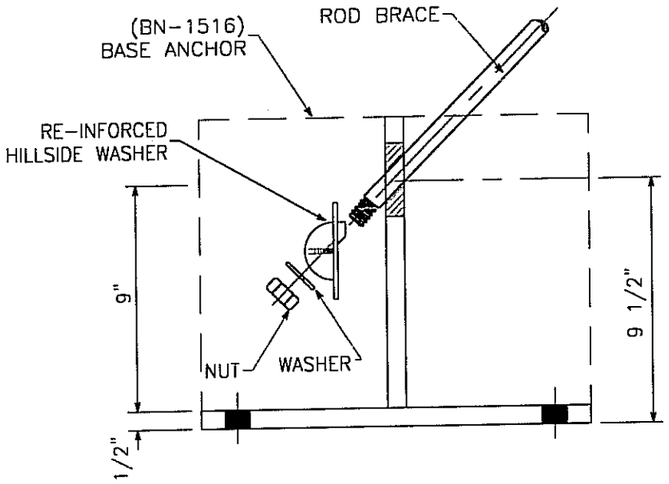
PROJECT:	DUNLAP 30
CUSTOMER:	CLOVIS STEEL STRUCTURES
LOCATION:	SQUAW VALLEY, CA 93675

DRAWN:	DATE:	JOB NO.	PAGE:
JDM	1/14/05	2792	PEB-4
CHECK:	DATE:		

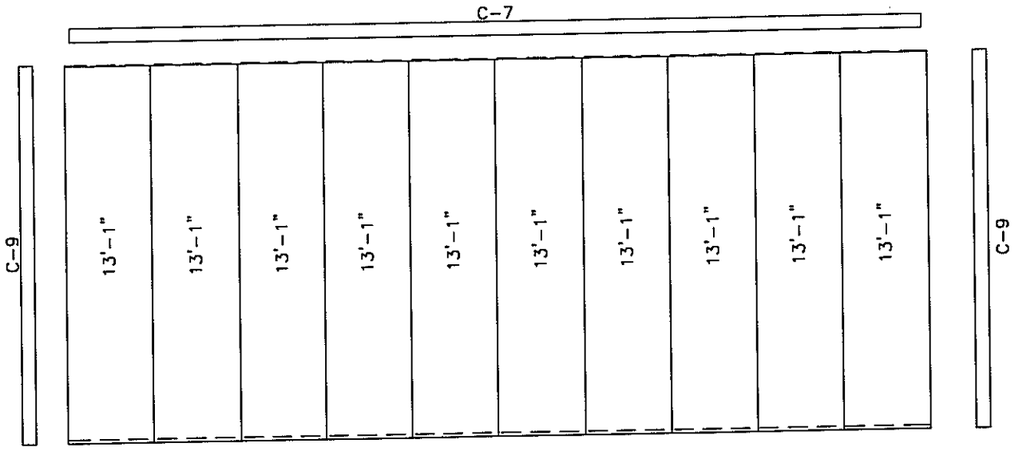
MEMBER TABLE LINE A	
MARK	SIZE
G-8	10Z30U12
G-9	10Z30U14
CB-3	ROD625



BACK SIDEWALL FRAMING: LINE A

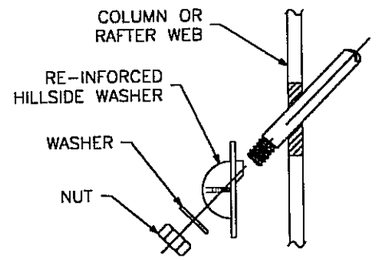


BASE ANCHOR DETAIL



BACK SIDEWALL SHEETING & TRIM: LINE A

PANELS: 26 Ga. HUSKY HI-RIB -



ROD BRACING DETAIL

Sheet 30 of 37

ERECTOR NOTE:
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Exp. 6/30/06
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1/19/05

REV.	DATE	DESCRIPTION
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REVISIONS

HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30
CUSTOMER:	CLOVIS STEEL STRUCTURES
LOCATION:	SQUAW VALLEY, CA 93675

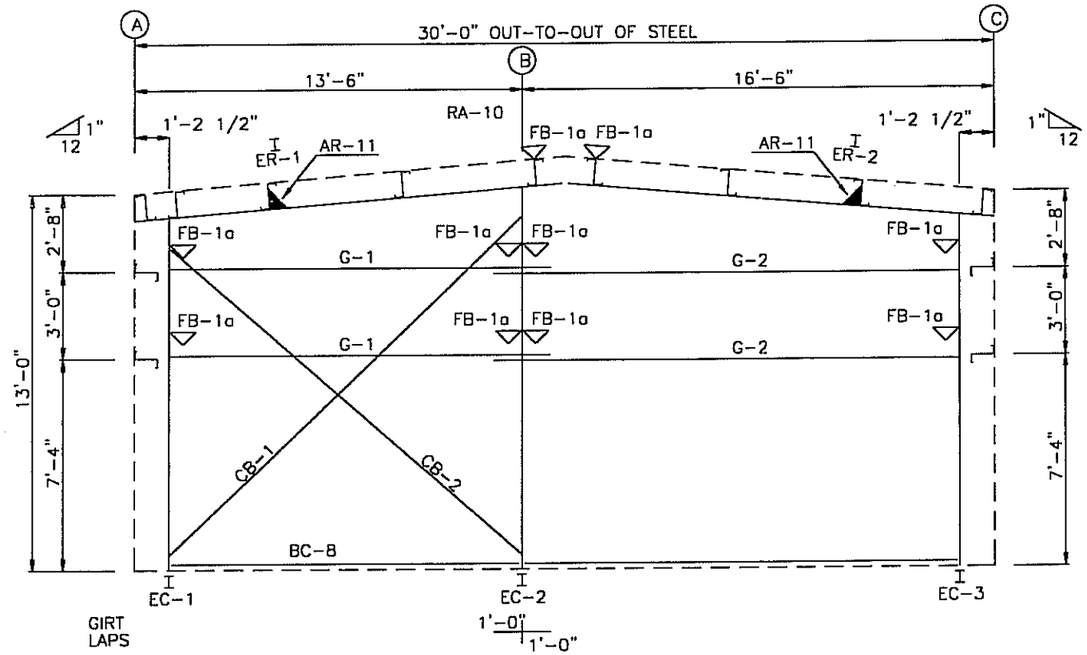
DRAWN: JDM	DATE: 1/14/05	JOB NO.	PAGE:
CHECK:	DATE:	2792	PEB-5

SPLICE PLATE
3/8"x6"x7 7/8"

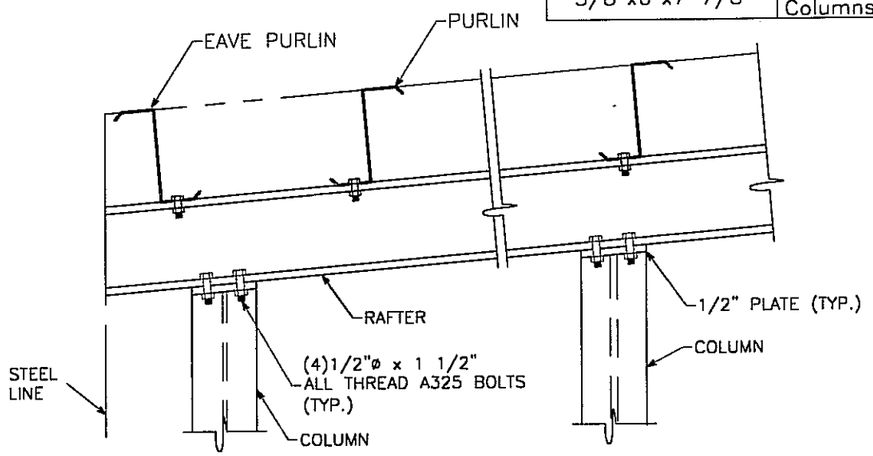
BOLT TABLE: FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LEN
ER-1-ER-2	4	A325	3/4"	1 3/4"
Columns	4	A325	1/2"	1 1/2"

MEMBER TABLE
LINE 1

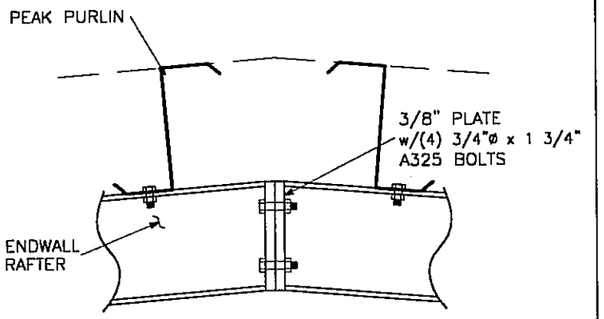
MARK	SIZE
EC-1	W8x10
EC-2	W8x10
EC-3	W8x10
ER-1	W8x10
ER-2	W8x10
G-1	8Z25U16
G-2	8Z25U16
CB-1	ROD625
CB-2	ROD625



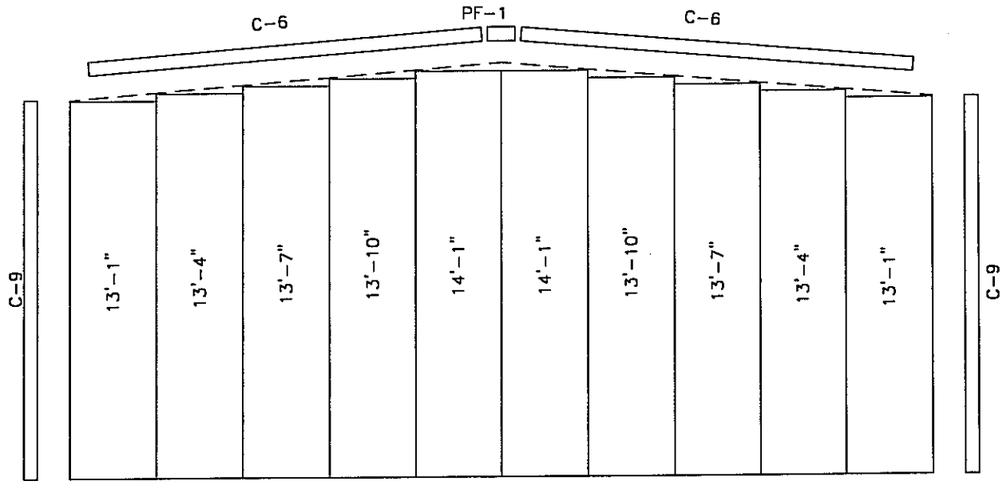
LEFT ENDWALL FRAMING: LINE 1



TYPICAL ENDWALL FRAMING DETAIL

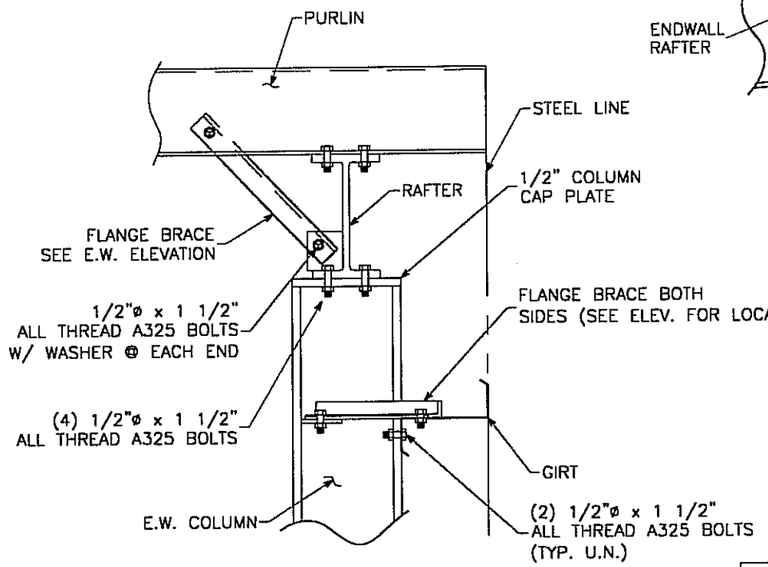


RIDGE CONNECTION
(WITHOUT CENTER COLUMN)



LEFT ENDWALL SHEETING & TRIM: LINE 1

PANELS: 26 Ga. HUSKY HI-RIB -



TYPICAL ENDWALL COLUMN CONN.

(BYPASS GIRT CONDITION)

Sheet 31 of 37

ERECTOR NOTE:
PIECE MARKS ARE LOCATED ON THE LEFT END OF MEMBER AS DETAILED. ERECT MEMBER SO PIECE MARK IS IN POSITION SHOWN ON ERECTION DRAWINGS.

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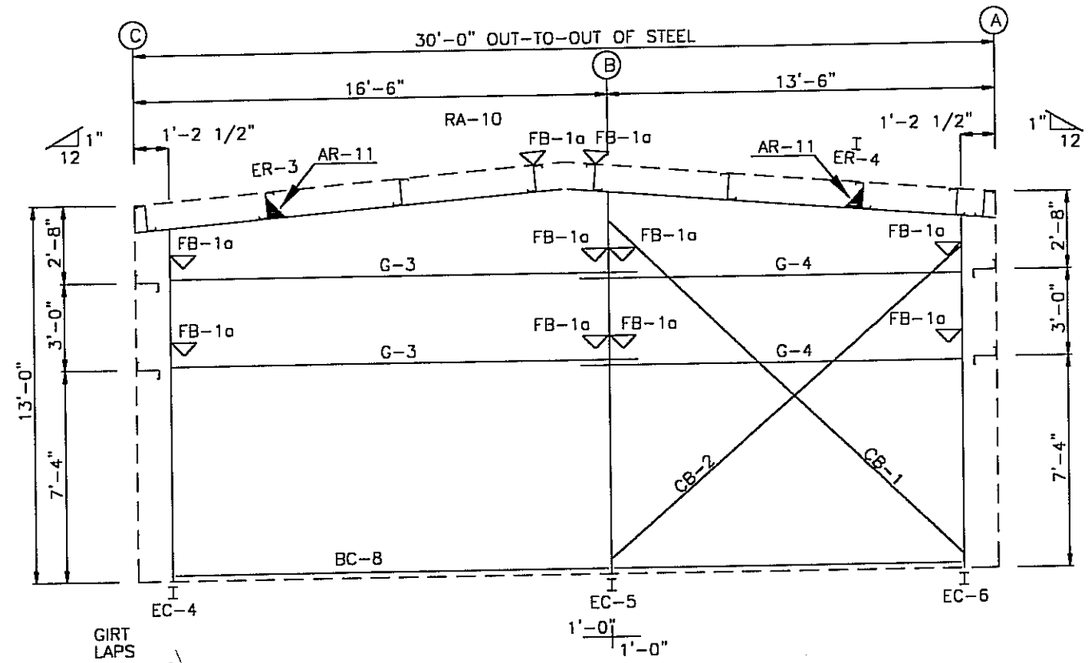
DRAWN: JDM	DATE: 1/14/05	JOB NO. 2792	PAGE: PEB-6
CHECK:	DATE:		

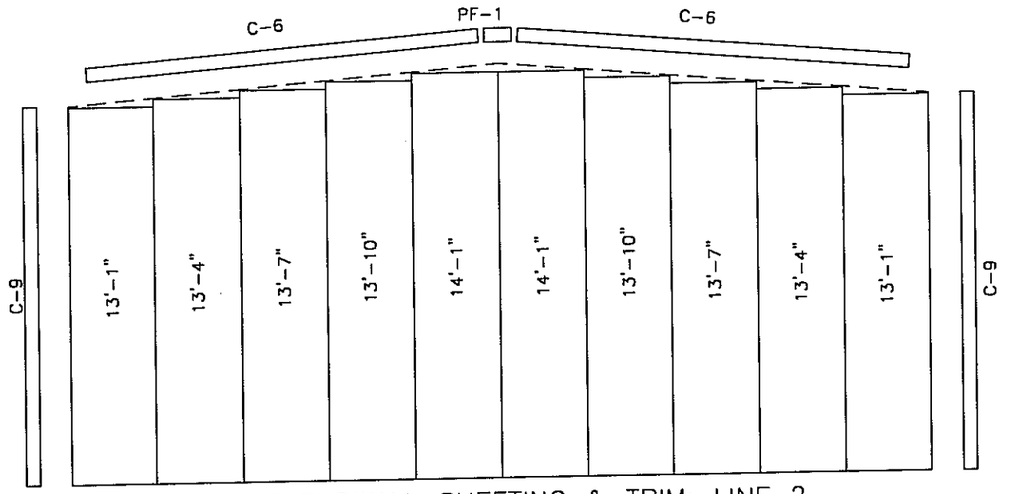
SPLICE PLATE	BOLT TABLE: FRAME LINE 2				
	LOCATION	QUAN	TYPE	DIA	LEN
3/8"x6"x7 7/8"	ER-3-ER-4 Columns	4	A325	3/4"	1 3/4"
		4	A325	1/2"	1 1/2"

MEMBER TABLE LINE 2	
MARK	SIZE
EC-4	W8x10
EC-5	W8x10
EC-6	W8x10
ER-3	W8x10
ER-4	W8x10
G-3	8Z25U16
G-4	8Z25U16
CB-1	ROD625
CB-2	ROD625



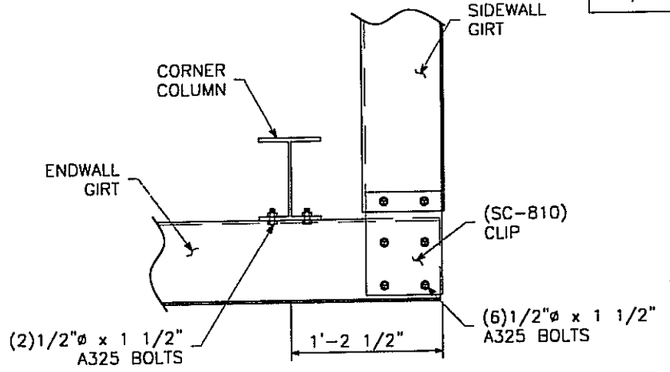
RIGHT ENDWALL FRAMING: LINE 2

4 1/2 + 10 + 3 = 17'



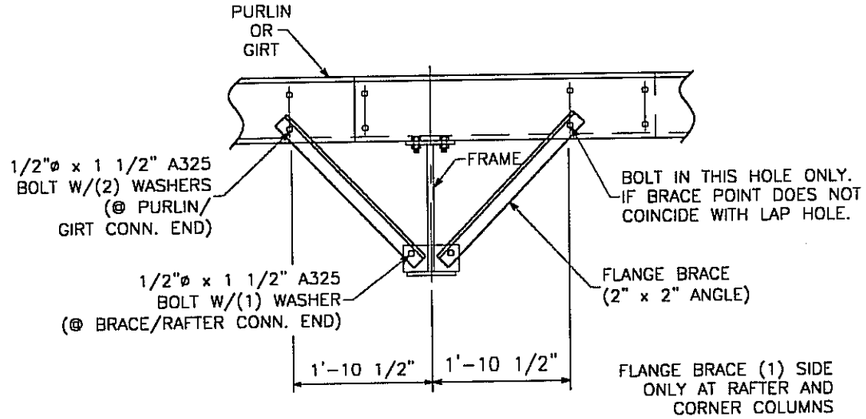
RIGHT ENDWALL SHEETING & TRIM: LINE 2

PANELS: 26 Ga. HUSKY HI-RIB -

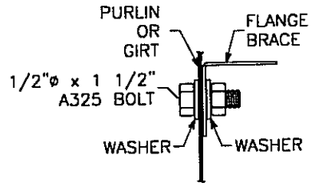


BEARING ENDWALL CORNER

WITH BYPASS ENDWALLS

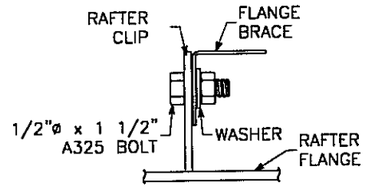


FLANGE BRACE DETAIL



WASHER PLACEMENT DETAIL

(CONNECTION @ PURLIN/GIRT)



WASHER PLACEMENT DETAIL

(CONNECTION @ RAFTER/COLUMN)

Sheet 32 of 37

ERECTOR NOTE:

PIECE MARKS ARE LOCATED ON THE LEFT END OF MEMBER AS DETAILED. ERECT MEMBER SO PIECE MARK IS IN POSITION SHOWN ON ERECTION DRAWINGS.

REGISTERED PROFESSIONAL ENGINEER MICHAEL R. LAPIERRE No. C66540 Exp. 6/30/06 CIVIL STATE OF CALIFORNIA

11/9/05

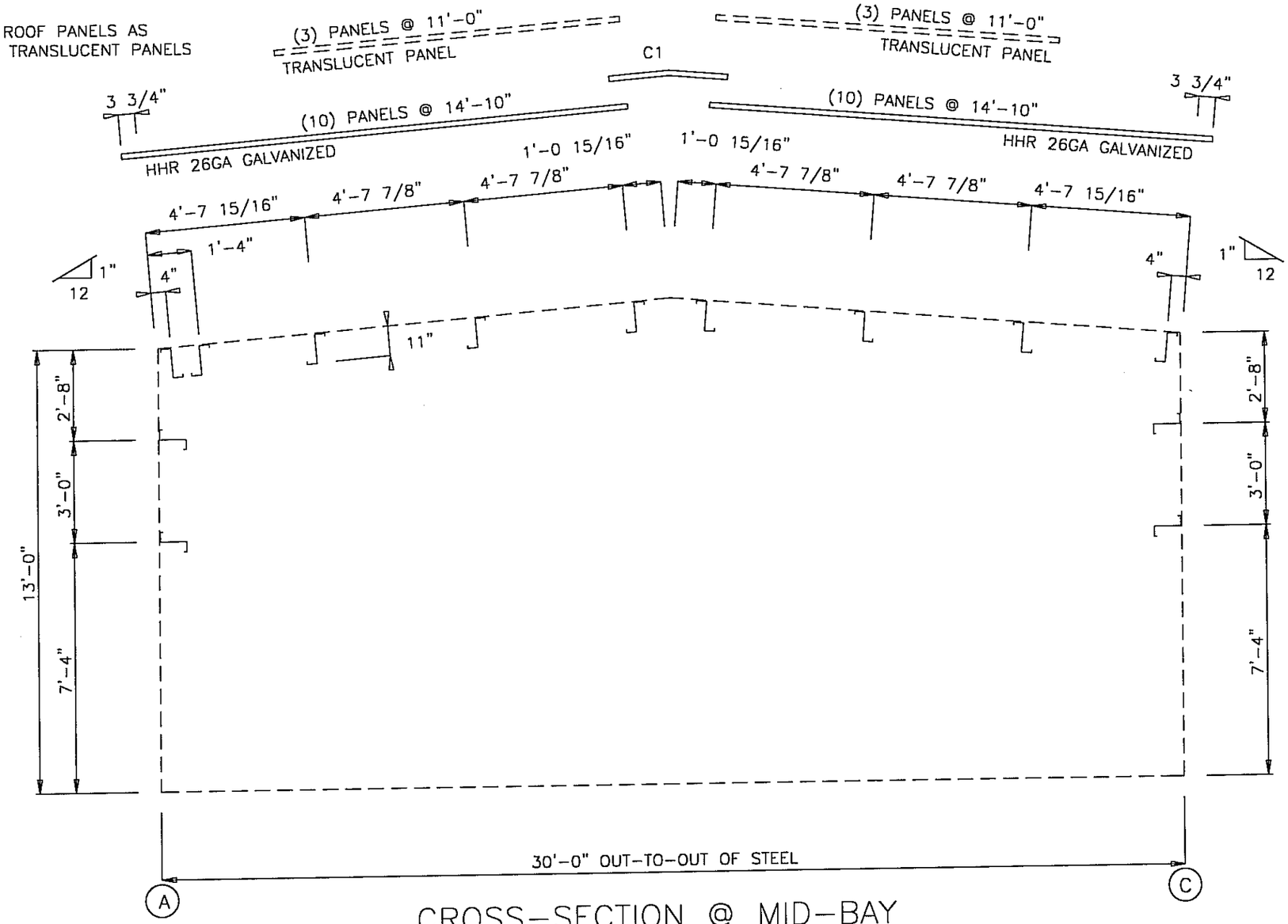
REV.	DATE	DESCRIPTION
A	1/19/05	FOR APPROVAL

REVISIONS

HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30
CUSTOMER:	CLOVIS STEEL STRUCTURES
LOCATION:	SQUAW VALLEY, CA 93675

DRAWN: JDM	DATE: 1/14/05	JOB NO. 2792	PAGE: PEB-7
CHECK:	DATE:		



CROSS-SECTION @ MID-BAY

Sheet 33 of 37

ERECTOR NOTE:
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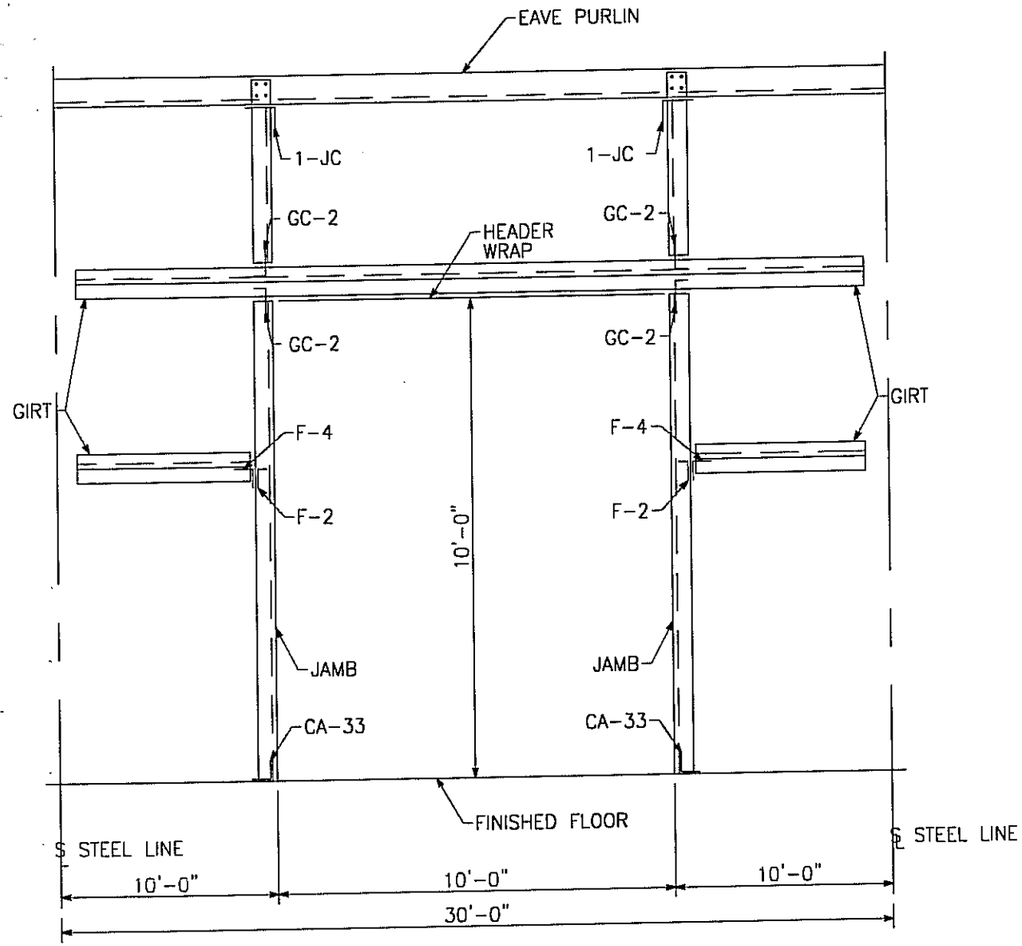
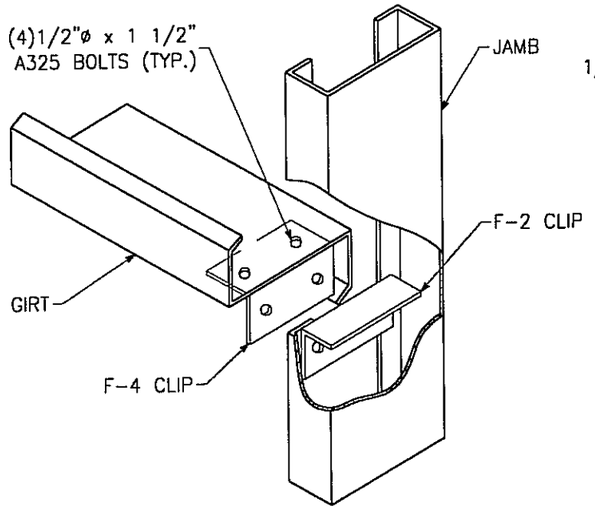
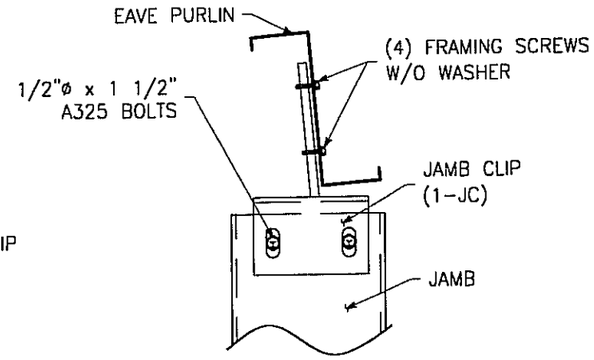
JDM	1/14/05	2792	PEB-8
CHECK:	DATE:		



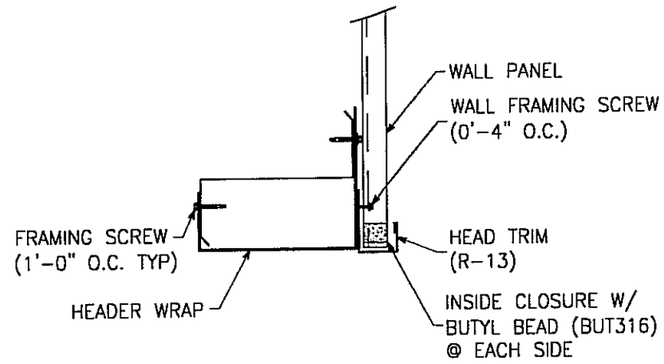
OVERHEAD DOOR FRAME ELEVATION



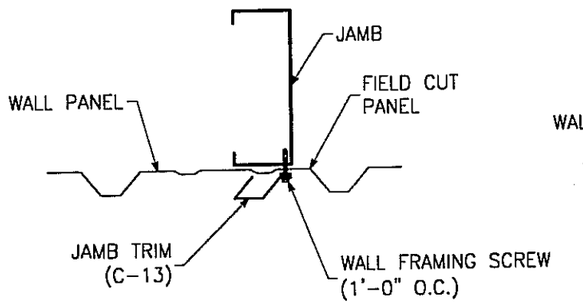
GIRT TO JAMB



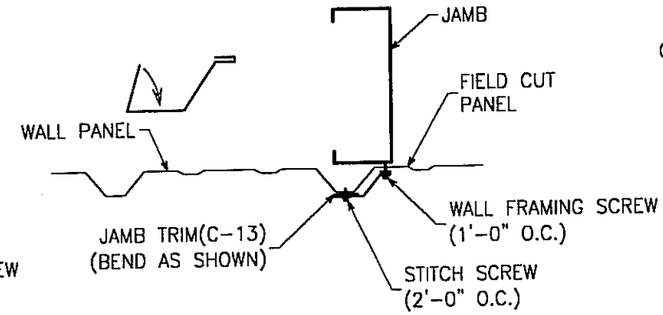
JAMB TO EAVE PURLIN



HEADER WRAP DETAIL



JAMB TRIM AT LOW RIB



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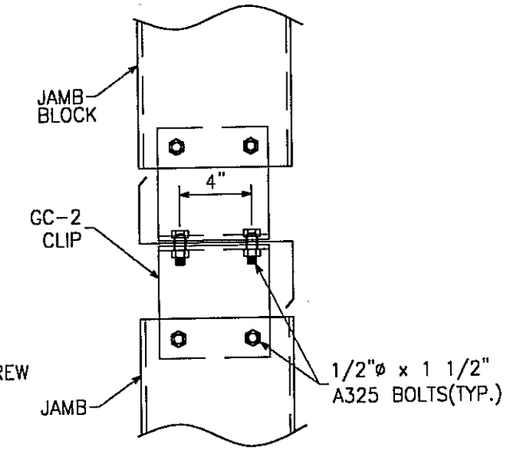

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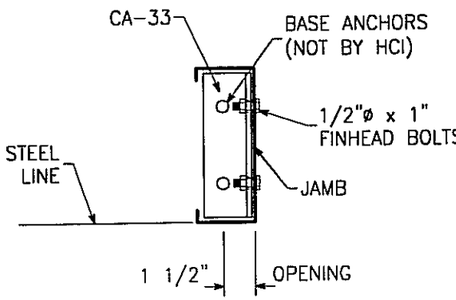


JAMB TO BASE

Sheet 34 of 37

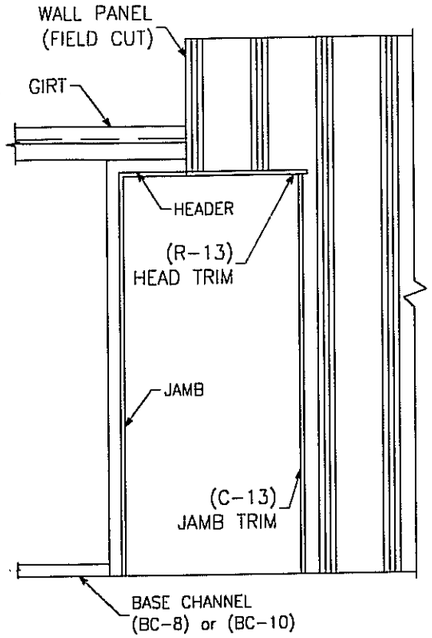
REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 6/30/06
CIVIL
STATE OF CALIFORNIA
1/19/05

REV.	DATE	DESCRIPTION
A	1/19/05	FOR APPROVAL

REVISIONS

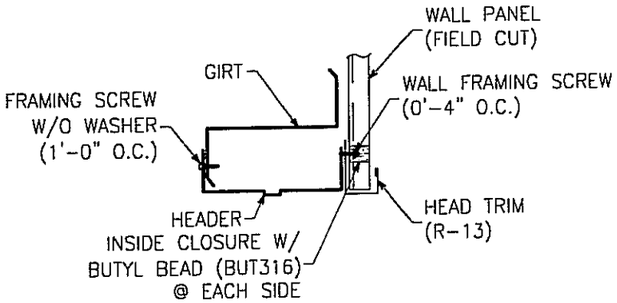
HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30	DRAWN: JDM	DATE: 1/14/05	JOB NO. 2792	PAGE: PEB-9
CUSTOMER:	CLOVIS STEEL STRUCTURES	CHECK:	DATE:		
LOCATION:	SQUAW VALLEY, CA 93675				



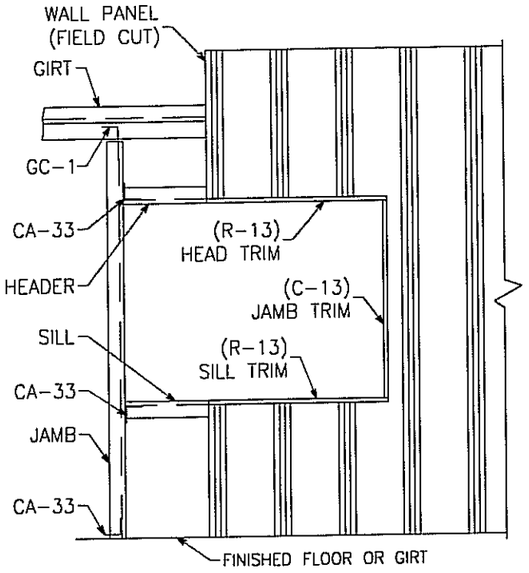
MAN DOOR ELEVATION

(1) REQUIRED

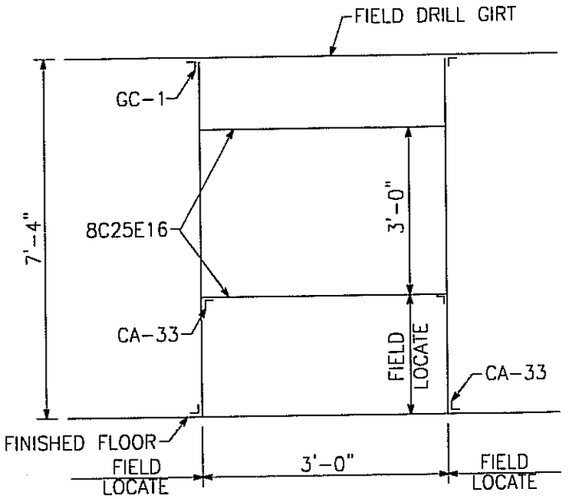


HEADER DETAIL

SILL SIMILAR

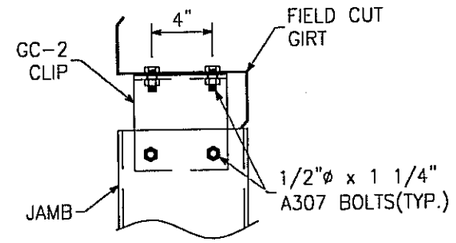


OPENING W/SILL ELEVATION

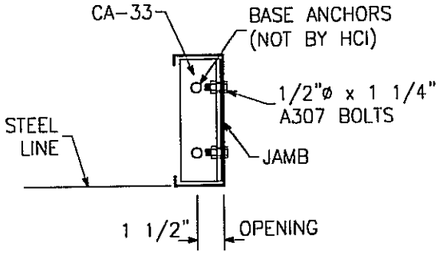


3'-0" x 3'-0" FRAMED OPENING

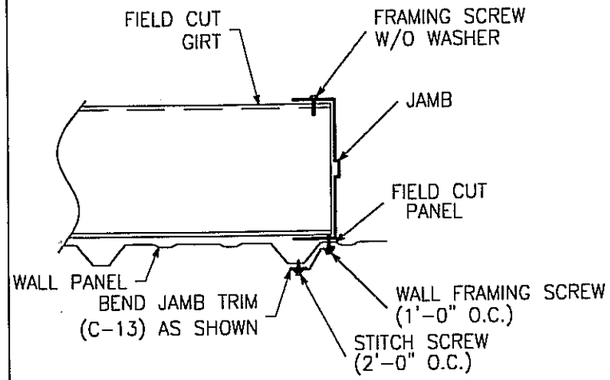
(2) REQUIRED



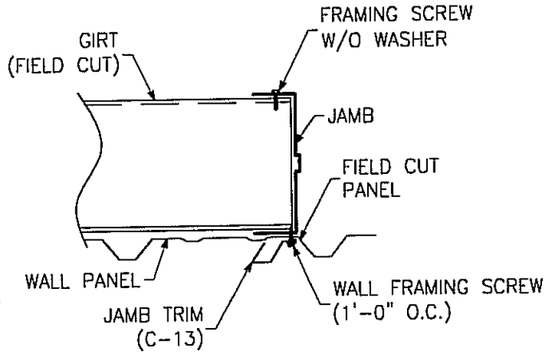
JAMB TO GIRT



JAMB TO BASE



JAMB TRIM AT HI-RIB



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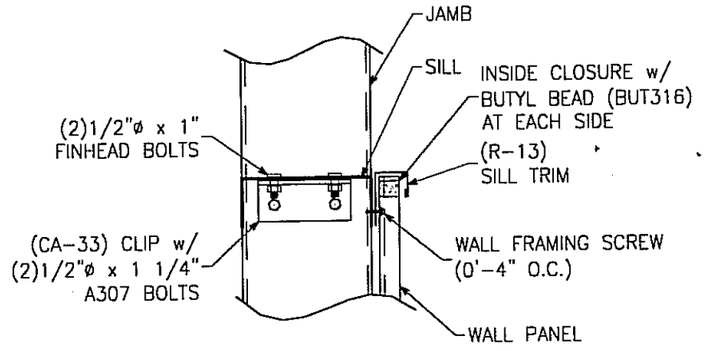


SILL DETAIL

Sheet 35 of 37

REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 6/30/06
CIVIL
STATE OF CALIFORNIA
11/19/05

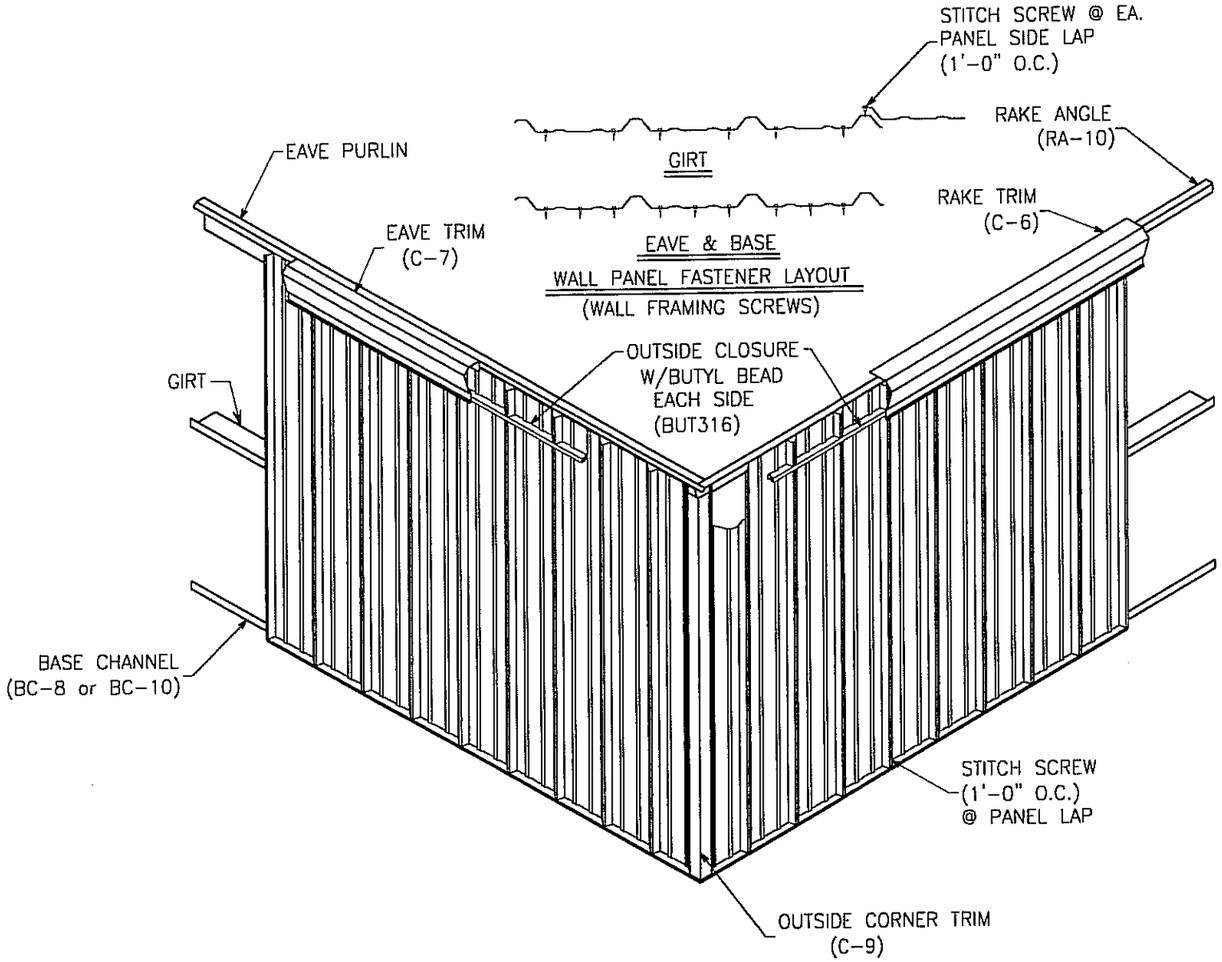
REV.	DATE	DESCRIPTION
A	1/19/05	FOR APPROVAL

REVISIONS

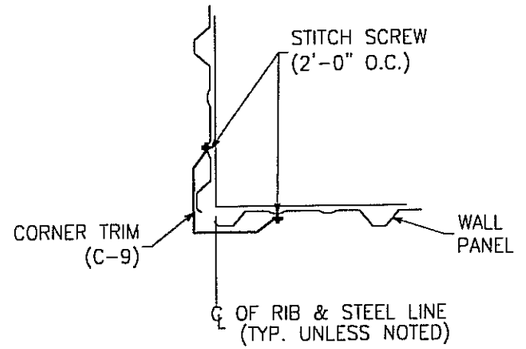
HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30
CUSTOMER:	CLOVIS STEEL STRUCTURES
LOCATION:	SQUAW VALLEY, CA 93675

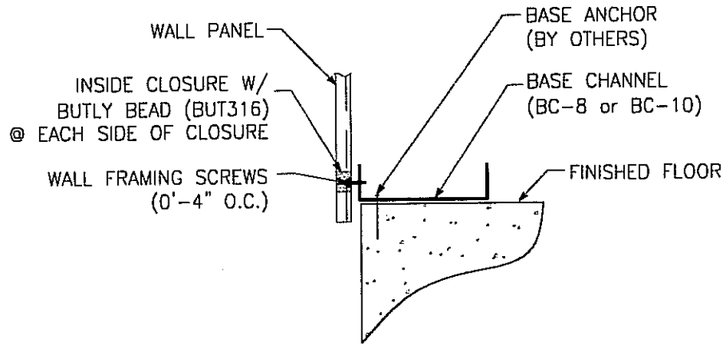
DRAWN:	DATE:	JOB NO.	PAGE:
JDM	1/14/05	2792	PEB-10
CHECK:	DATE:		



HHR WALL PANEL LAYOUT



OUTSIDE CORNER TRIM



BASE CHANNEL DETAIL

Sheet 36 of 37

REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp. 6/30/06
CIVIL
STATE OF CALIFORNIA
1/19/05

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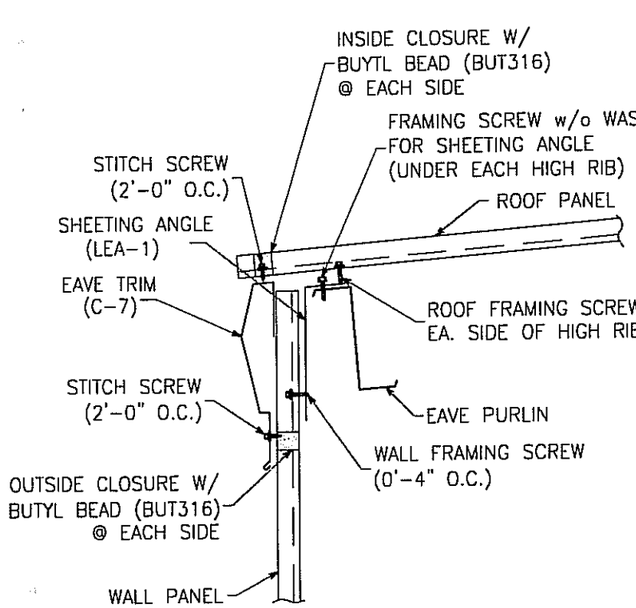
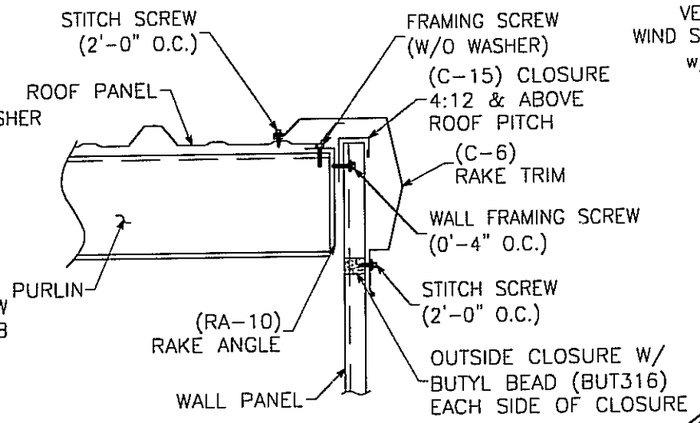
LOCATION:	SQUAW VALLEY, CA 93675

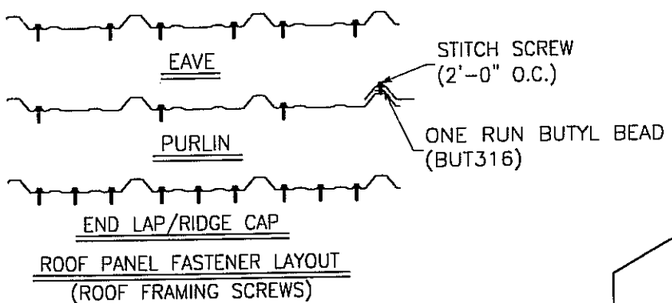
DRAWN:	DATE:	JOB NO.	PAGE:
JDM	1/14/05	2792	PEB-11
CHECK:	DATE:		



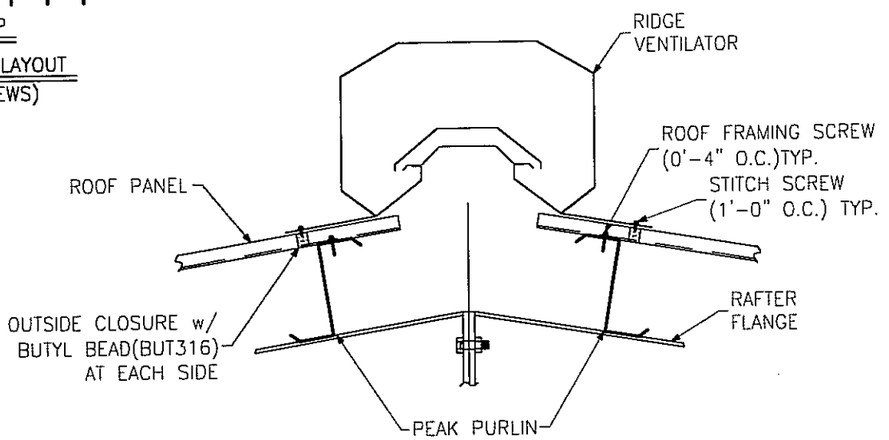
EAVE TRIM DETAIL



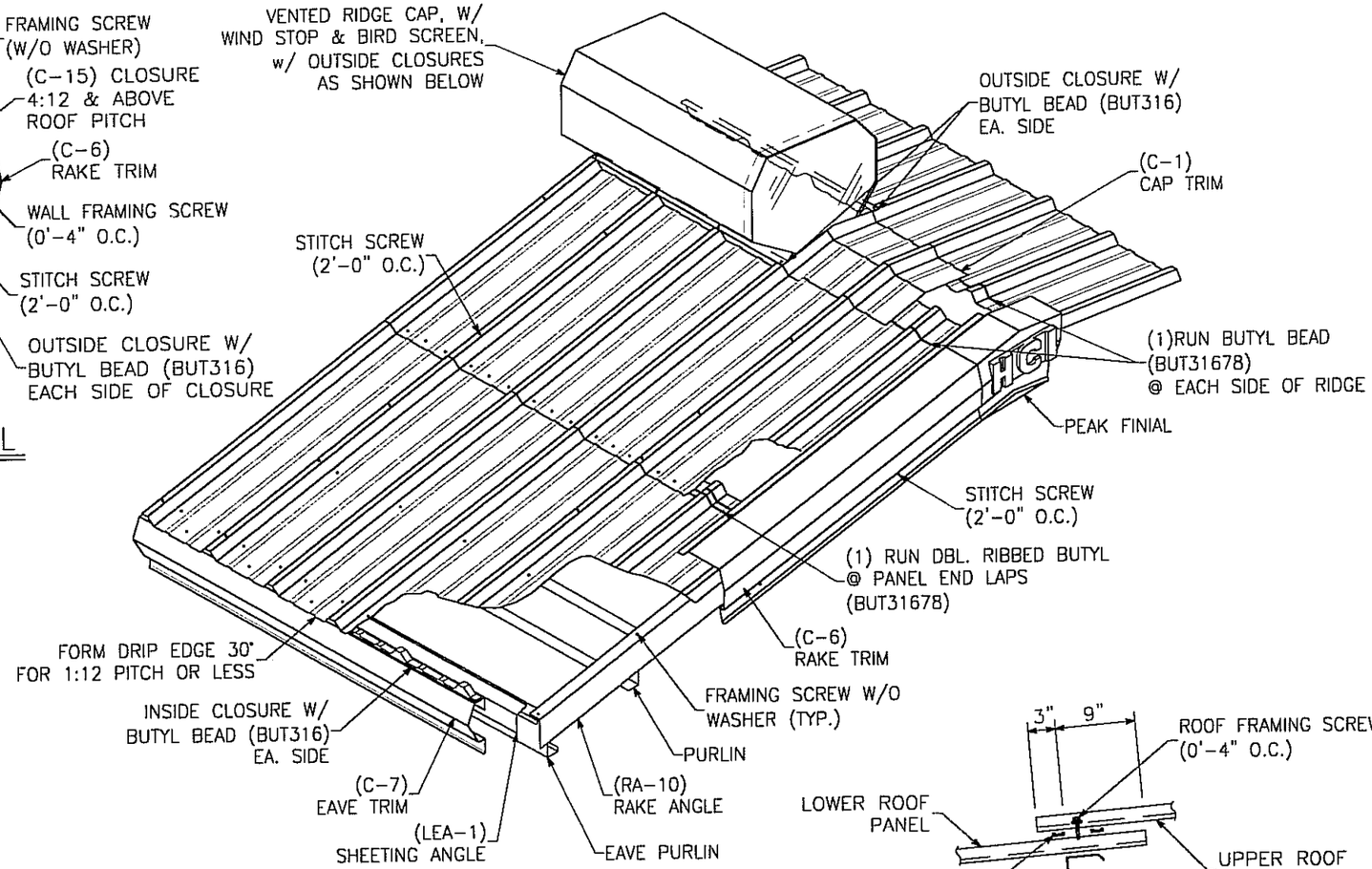
RAKE TRIM DETAIL



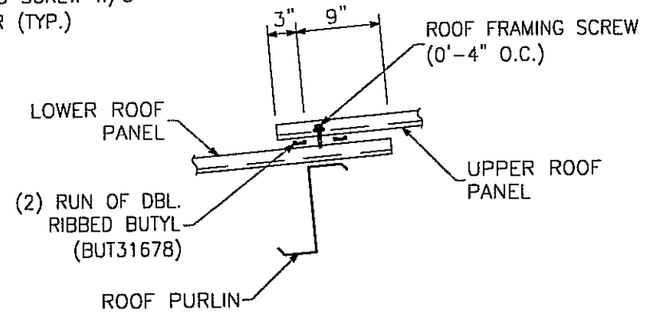
ROOF PANEL FASTENER LAYOUT
(ROOF FRAMING SCREWS)



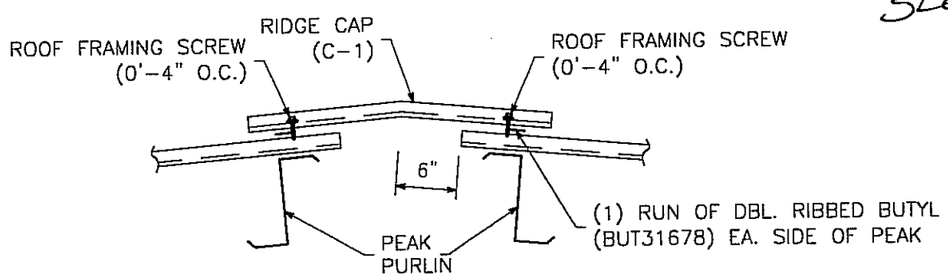
RIDGE VENTILATOR DETAIL



HHR ROOF PANEL LAYOUT



PANEL ENDLAP DETAIL



RIDGE CAP DETAIL

Sheet 37 of 37

REGISTERED PROFESSIONAL ENGINEER
MICHAEL R. LAPIERRE
No. C66540
Exp 6/30/06
CIVIL
STATE OF CALIFORNIA
1/19/05

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REVISIONS

HCI STEEL BUILDING SYSTEMS INC.

PROJECT:	DUNLAP 30
CUSTOMER:	CLOVIS STEEL STRUCTURES
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DRAWN:	DATE:	JOB NO.	PAGE:
JDM	1/14/05	2792	PEB-12
CHECK:	DATE:		