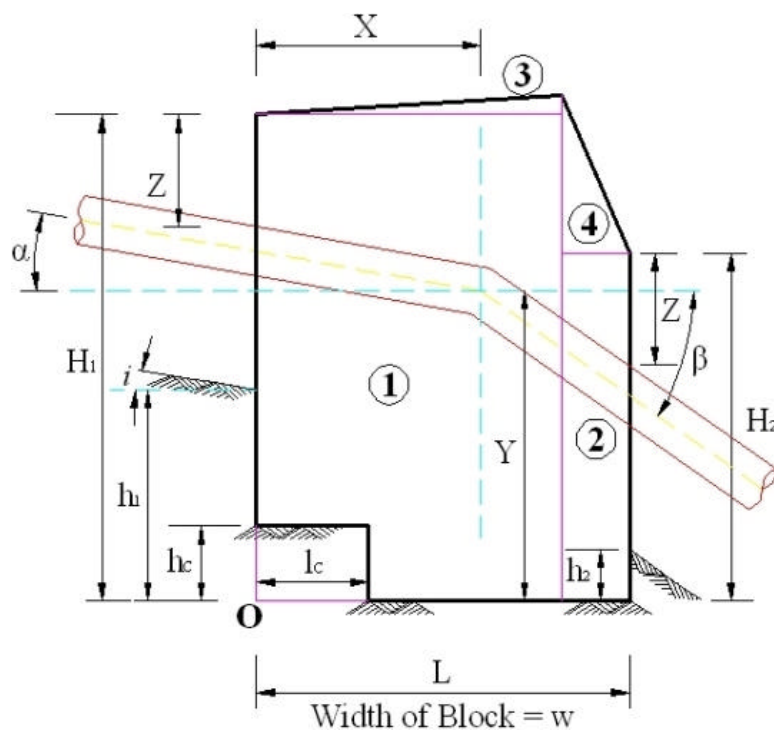


**Design of an Anchor Block – Type 2**

Model of Type 2 Anchor Block:

**Example:** Anchor block of Chandra Jyoti MHP, Kavre

$$H_1 = 1.3 \text{ m} \quad L = 1.0 \text{ m}$$

$$w = 0.9 \text{ m} \quad Z = 0.3 \text{ m}$$

$$h_1 = 0.6 \text{ m} \quad h_2 = 0.3 \text{ m}$$

$$X = 0.6 \text{ m} \quad i = 10^\circ$$

$$l_c = 0.3 \text{ m} \quad h_c = 0.2 \text{ m}$$

$$a = 10^\circ \quad b = 35^\circ$$

$$\text{Pipe Diameter, } d = 130 \text{ mm}$$

$$\text{Pipe Thickness, } t = 4 \text{ mm}$$

$$\text{Discharge, } Q = 20 \text{ lps}$$

$$\text{Gross Head, } h_{gross} = 42 \text{ m}$$

$$\text{Surge Head, } h_{surge} = 95 \text{ m}$$

$$\text{Distance to u/s support pier, } L_{2u} = 4 \text{ m}$$

$$L_{1u} = 2 \text{ m}$$

$$\text{Distance to d/s support pier, } L_{2d} = 4 \text{ m}$$

$$L_{1d} = 2 \text{ m}$$

$$\text{Distance to u/s expansion joint} = 18 \text{ m}$$

$$L_{4u} = 18 \text{ m}$$

$$\text{Number of piers at upstream} = 4$$

Relation Used for Area Calculation:

$A_1$	$H_1 N$	$1.08 \text{ m}^2$
$A_2$	$H_2(L - N)$	$0.16 \text{ m}^2$
$A_3$	$\frac{1}{2} N(H_2 + M - H_1)$	$0.03 \text{ m}^2$
$A_4$	$\frac{1}{2} M(L - N)$	$0.04 \text{ m}^2$

Relation Used for Centre of Gravity Calculation:

$X_1$	$\frac{1}{2} N$	$0.42 \text{ m}$
$X_2$	$\frac{1}{2} (L + N)$	$0.92 \text{ m}$
$X_3$	$\frac{2}{3} N$	$0.55 \text{ m}$
$X_4$	$\frac{1}{3} (L + 2N)$	$0.89 \text{ m}$

Relation Used for Calculating Deduction Volume Occupied by Penstock:

$V_p$	$\frac{p}{4} l_u (d_u + 2t)^2 + \frac{p}{4} l_d (d_d + 2t)^2$	$0.02 \text{ m}^3$
Where	$Y = H_1 - Z - \frac{d + 2t}{2 \cos a} - X \tan a$ $H_2 = Y - (L - X) \tan b + \frac{d + 2t}{2 \cos b} + Z$ $M = Y - H_2 + \left[ \frac{5Z}{3} + \frac{d + 2t}{2 \cos(90 - a - q)} \right] \sin q$	$Y = 0.82 \text{ m}$ $H_2 = 0.93 \text{ m}$ $M = 0.44 \text{ m}$

$N = X + \left[ \frac{5Z}{3} + \frac{d + 2t}{2 \cos(90 - a - q)} \right] \cos q$ $q = \frac{1}{2} (180 - a - b)$ $l_u = \frac{X}{\cos a}$ $l_d = \frac{L - X}{\cos b}$	$N = 0.83 \text{ m}$ $q = 67.5^\circ$ $l_u = 0.61 \text{ m}$ $l_d = 0.49 \text{ m}$
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**Further Design Procedure for the Block,**

$$\begin{aligned} \text{Total head, } h_{total} &= h_{gross} + h_{surge} &= 42 + 95 \\ & &= 137 \text{ m} \end{aligned}$$

$$\begin{aligned} \text{Block volume excluding pipe volume, (V)} &= w (A_1 + A_2 + A_3 + A_4 - l_c h_c) - V_p \\ &= 0.9 (1.08 + 0.16 + 0.03 + 0.04 - 0.3 \times 0.2) - 0.02 \text{ m}^3 \\ &= 1.10 \text{ m}^3 \end{aligned}$$

$$\text{Unit weight of concrete, (} g_{concrete} \text{)} = 22 \text{ kN/m}^3$$

$$\begin{aligned} \text{Weight of block, } W_B &= V g_{concrete} \\ &= 1.10 \times 22 \\ &= 24.20 \text{ kN} \end{aligned}$$

$$\text{Unit weight of steel, (} g_{steel} \text{)} = 77 \text{ kN/m}^3$$

$$\begin{aligned} \text{Weight of pipe, } W_P &= p(d + t)t g_{steel} \\ &= p \times 0.134 \times 0.004 \times 77 \\ &= 0.13 \text{ kN/m} \end{aligned}$$

$$\text{Unit weight of water, (} g_{water} \text{)} = 9.81 \text{ kN/m}^3$$

$$\begin{aligned} \text{Weight of water, } W_W &= \frac{pd^2}{4} \times g_{water} \\ &= \frac{p(0.13)^2}{4} \times 9.81 \\ &= 0.13 \text{ kN/m} \end{aligned}$$

$$W_P + W_W = 0.26 \text{ kN/m}$$

**Calculation for relevant forces:**

$$\begin{aligned} 1. \quad F_{1u} &= (W_P + W_W)L_{1u} \cos a \\ &= 0.26 \times 2 \times \cos 10^\circ = 0.51 \text{ kN} \end{aligned}$$

$$\begin{aligned} 2. \quad F_{1d} &= (W_P + W_W)L_{1d} \cos b \\ &= 0.26 \times 2 \times \cos 35^\circ = 0.43 \text{ kN} \end{aligned}$$

$$\begin{aligned} 3. \quad \text{Frictional force per support pier:} \\ &= \pm f(W_P + W_W)L_{2u} \cos a \\ &\text{Where } f = 0.6 \text{ for steel on concrete} \\ &= \pm 0.6 \times 0.26 \times 4 \times \cos 10^\circ = \pm 0.61 \text{ kN per support pier} \end{aligned}$$

Since there are 4 support piers then,

$$F_{2u} = \pm 0.61 \times 4 = \pm 2.46 \text{ kN}$$

Note that  $F_{2d}$  is zero since an expansion joint is located immediately downstream of the anchor block.

$$\begin{aligned} 4. F_3 &= 2g_{\text{water}} h_{\text{total}} \frac{pd^2}{4} \sin\left(\frac{b-a}{2}\right) \\ &= 2 \times 9.81 \times 137 \times \frac{p \times 0.13^2}{4} \sin\left(\frac{35-10}{2}\right) \\ &= 7.72 \text{ kN} \end{aligned}$$

$$\begin{aligned} 5. F_{4u} &= W_p L_{4u} \sin a \\ &= 0.13 \times 18 \times \sin 10^\circ = 0.41 \text{ kN} \end{aligned}$$

But  $F_{4u}$  is insignificant since  $a$  is less than  $20^\circ$  and could have been ignored.  $\therefore F_{4u} \sim 0$   
 $F_{4d}$  is negligible since an expansion joint is placed immediately downstream of the anchor block, i.e.,  $L_{4d} \sim 0$  and therefore  $F_{4d} \sim 0$

$$\begin{aligned} 6. F_5 &= Ea\Delta T p(d+t)t \\ \text{Since the expansion joint is installed between the blocks,} \\ F_5 &= 0 \end{aligned}$$

$$\begin{aligned} 7. F_6 &= \pm 100d \\ &= \pm 100 \times 0.13 = \pm 13 \text{ kN} \end{aligned}$$

$$\begin{aligned} 8. F_7 &= g_{\text{water}} h_{\text{total}} p(d+t)t \\ F_{7u} &= 9.81 \times (137 - 18 \sin a) \times p \times 0.134 \times 0.004 \\ &= 2.21 \text{ kN} \\ F_{7d} &= 9.81 \times 137 \times p \times 0.134 \times 0.004 \\ &= 2.26 \text{ kN} \end{aligned}$$

Note that as discussed earlier the resultant of these forces is insignificant.

$$\begin{aligned} 9. F_8 &= \frac{8Q^2}{pd^2} \sin\left(\frac{b-a}{2}\right) \\ &= \frac{8 \times 0.02^2}{p \times 0.13^2} \sin\left(\frac{35-10}{2}\right) \\ &= 0.01 \text{ kN} \end{aligned}$$

Note that as discussed earlier, this force is insignificant.

$$10. F_9 = g_{\text{water}} h_{\text{total}} \frac{p}{4} (d_u^2 - d_d^2)$$

Since the pipe diameter does not change,

$$F_9 = 0$$

11. Soil force,  $F_{10}$

$$\text{Unit weight of concrete, } (\gamma_{\text{soil}}) = 20 \text{ kN/m}^3$$

$$\text{For stiff clay and stiff sandy clay } (\phi) = 30^\circ$$

$$K_a = \frac{\cos i - \sqrt{\cos^2 i - \cos^2 f}}{\cos i + \sqrt{\cos^2 i - \cos^2 f}} = 0.355$$

$$F_{10} = K_a w \frac{g_{soil} h_1^2}{2} \cos i$$

But in this example,

$$h_1 - h_2 = 0.6 - 0.3 = 0.3 < 1, \text{ So } F_{10} \text{ is insignificant, therefore } F_{10} = 0$$

### Resolution of forces:

$$a = 10^\circ, b = 35^\circ$$

Forces (kN)	X – component (kN) → +	Y – component (kN) ↓ +
$F_{1u} = 0.51$	$- F_{1u} \sin a = -0.09$	$+ F_{1u} \cos a = +0.50$
$F_{1d} = 0.43$	$- F_{1u} \sin b = -0.25$	$+ F_{1u} \cos b = +0.35$
$F_{2u} = \pm 2.46$	$\pm F_{2u} \cos a = \pm 2.42$ Negative during <sup>1</sup> expansion Positive during <sup>2</sup> contraction	$\pm F_{2u} \sin a = \pm 0.43$ Negative during <sup>1</sup> expansion Positive during <sup>2</sup> contraction
$F_3 = 7.72$	$+ F_3 \sin\left(\frac{b+a}{2}\right) = +2.95$	$- F_3 \cos\left(\frac{b+a}{2}\right) = -7.13$
$F_6 = \pm 13$	$\pm F_6 (\cos a - \cos b) = \pm 2.15$ Positive during <sup>1</sup> expansion Negative during <sup>2</sup> contraction	$\pm F_6 (\sin b - \sin a) = \pm 5.20$ Positive during <sup>1</sup> expansion Negative during <sup>2</sup> contraction
$F_{7u} = 2.21$	$+ F_{7u} \cos a = +2.18$	$+ F_{7u} \sin a = +0.38$
$F_{7d} = 2.26$	$- F_{7d} \cos b = -1.85$	$- F_{7d} \sin b = -1.30$
$F_8 = 0.01$	$+ F_8 \sin\left(\frac{b+a}{2}\right) \sim 0$	$- F_8 \cos\left(\frac{b+a}{2}\right) = -0.01$
$W_B = 24.20$	0	+ 24.20
SUM	$\sum H = +2.67$ <sup>1</sup> Expansion $\sum H = +3.21$ <sup>2</sup> Contraction	$\sum V = +21.76$ <sup>1</sup> Expansion $\sum V = +12.22$ <sup>2</sup> Contraction

Note that forces are positive in X-direction is towards the right and Y-direction downwards.

$$\begin{aligned} \text{Sum of horizontal forces that act at the bend,} &= \sum H - F_{10x} \\ &\begin{aligned} &^1\text{Expansion case} &= +2.67 - 0 &= +2.67 \text{ kN} \rightarrow \\ &^2\text{Contraction case} &= +3.21 - 0 &= +3.21 \text{ kN} \rightarrow \end{aligned} \\ \text{Sum of vertical forces that act at the bend,} &= \sum V - F_{10y} - W_B \\ &\begin{aligned} &^1\text{Expansion case} &= +21.76 - 0 - 24.20 &= -2.44 \text{ kN} \uparrow \\ &^2\text{Contraction case} &= +12.22 - 0 - 24.20 &= -11.98 \text{ kN} \uparrow \end{aligned} \end{aligned}$$

Calculate for the centre of gravity of the block from the upstream face of the block taking the moment of mass. The effect of the pipe passing through the block is considered negligible, so need not be calculated.

$$\begin{aligned} \bar{X} &= \frac{\sum A_i X_i}{\sum A_i} \times \frac{w \mathbf{r}_{concrete}}{w \mathbf{r}_{concrete}} \\ &= \frac{1.08 \times 0.42 + 0.16 \times 0.92 + 0.03 \times 0.55 + 0.04 \times 0.89 - 0.3 \times 0.2 \times 0.15}{1.08 + 0.16 + 0.03 + 0.04 - 0.3 \times 0.2} = 0.51 \text{ m} \end{aligned}$$

∴ The weight of the block  $W_B$  acts 0.51 m from point O.

A force diagram on the block is as shown in Figure;

**Checking for Safety of the Block:**

1. Checking safety against overturning

<sup>1</sup>Expansion Case

Taking sum of moment about point O with clockwise moments as positive:

$$\begin{aligned}\sum M \text{ at O,} \\ &= 24.20 \times 0.51 + 2.67 \times 0.82 - 2.44 \times 0.60 \\ &= 13.06 \text{ kN-m}\end{aligned}$$

$$\frac{\sum M}{\sum V} = \frac{13.06}{21.76} = 0.60 \text{ m}$$

$$e = \left| \frac{L}{2} - \frac{\sum M}{\sum V} \right| = |0.50 - 0.60| = 0.1$$

$$e_{\text{allowable}} = \frac{L_{\text{base}}}{6} = \frac{1}{6} = 0.17$$

$$\therefore e < e_{\text{allowable}} \quad \text{OK}$$

<sup>2</sup>Contraction Case

Taking sum of moment about point O with clockwise moments as positive:

$$\begin{aligned}\sum M \text{ at O,} \\ &= 24.20 \times 0.51 + 3.21 \times 0.82 - 11.96 \times 0.60 \\ &= 7.80 \text{ kN-m}\end{aligned}$$

$$\frac{\sum M}{\sum V} = \frac{7.80}{12.22} = 0.64 \text{ m}$$

$$e = \left| \frac{L}{2} - \frac{\sum M}{\sum V} \right| = |0.50 - 0.64| = 0.14$$

$$e_{\text{allowable}} = \frac{L_{\text{base}}}{6} = \frac{1}{6} = 0.17$$

$$\therefore e < e_{\text{allowable}} \quad \text{OK}$$

Since  $e < e_{\text{allowable}}$  for both cases, the structure is safe against overturning.

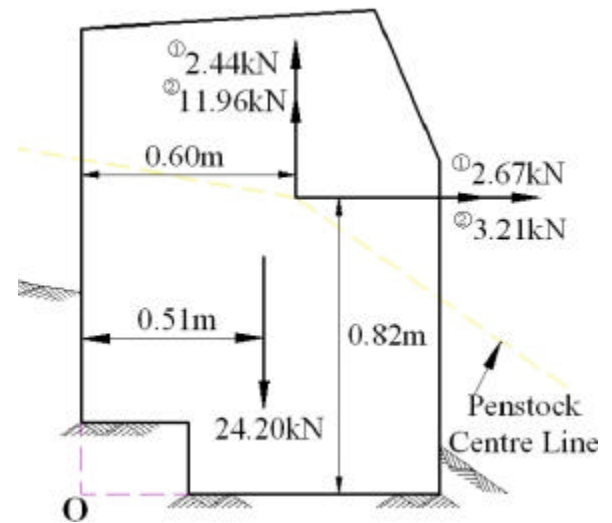
2. Checking safety on bearing capacity

For stiff clay allowable bearing pressure is  $200 \text{ kN/m}^2$

<sup>1</sup>Expansion Case

$$P_{\text{base}} = \frac{\sum V}{A_{\text{base}}} \left( 1 + \frac{6e}{L_{\text{base}}} \right) = \frac{21.76}{1 \times 0.9} \left( 1 + \frac{6 \times 0.1}{1} \right) = 38.68 \text{ kN/m}^2$$

$$P_{\text{base}} < P_{\text{allowable}} \quad \text{OK}$$



<sup>2</sup>Contraction Case

$$P_{base} = \frac{\sum V}{A_{base}} \left( 1 + \frac{6e}{L_{base}} \right) = \frac{12.22}{1 \times 0.9} \left( 1 + \frac{6 \times 0.14}{1} \right) = 24.98 \text{ kN/m}^2$$

$$P_{base} < P_{allowable} \quad \text{OK}$$

In both cases  $P_{base} < P_{allowable} = 200 \text{ kN/m}^2$ ,  $\therefore$  the structure is safe against sinking.

## 3. Checking safety against sliding

<sup>1</sup>Expansion Case

$$\sum H < m \sum V$$

$m = 0.5$  for concrete/masonry on soil

$$2.67 \text{ kN} < 0.5 \times 21.76 \text{ kN}$$

$$2.67 \text{ kN} < 10.88 \text{ kN} \quad \text{OK}$$

<sup>2</sup>Contraction Case

$$\sum H < m \sum V$$

$m = 0.5$  for concrete/masonry on soil

$$3.21 \text{ kN} < 0.5 \times 12.22 \text{ kN}$$

$$3.21 \text{ kN} < 6.11 \text{ kN} \quad \text{OK}$$

Since,  $\sum H < m \sum V$  in both cases the structure is safe against sliding.

$\therefore$  The anchor block is stable.